



Alfa Laval Inc.
5400 International Trade Dr.
Richmond, VA 23231

Date: 8/12/2015

Rev.: 1

Title: Data Book Index

Doc. No.: 85480-4-DBI

Page 1 of 1

Customer: Viridia B2X, LLC

SU Order No: 85480-40

Model : TS6-MFG

Serial No: 30116-93128

Customer PO: 148

Item No: HE-2853

PLATE HEAT EXCHANGER DATABOOK

Section 1: GA Drawing / Plate Hanging List / Technical Specification Sheet

Section 2: ASME Data Report / Code Plate Copy / Certificate of Conformance

Section 3: Pressure Part Material Certificates

- Frame and Pressure Plate MTR
- Channel Plate MTR
- Tightening Bolt and Nut (Representative MTR)

Section 4: Standard Hydrostatic Test Certificate

Section 5: Calculations

- Stress Calculations
- Lifting Device Calculations

Prepared By: Peter Scholtes

Approved By: Peter Scholtes

Rev1: Revised for change in design



Alfa Laval, Plate Heat Exchanger Channel Plate Installation Description

2015-08-12

Customer:	Viridia B2X, LLC	SU Order No:	85480-40
Model :	TS6-MFG	Serial No:	30116-93128
Customer PO:	148	Item No:	HE-2853

Plate material and Thickness: SA-240-316 0.60 mm

A Dimension: 47 mm

	Hot side	Cold side
Grouping:	1*4H	1*5H
Sealing material:	EPDMP CLIP-ON	EPDMP CLIP-ON
Port Locations:	S4 -> S3	S2 -> S1

Connection material: SA-240-316 SA-240-316

Port hole with flow on the gasketed side: U

Port hole sealed with O-ring: O

Plates are assembled with the gasket side facing the frame plate.

Plate no.	Plate code no.	Plate Pattern		Punched corner of the plate				Flow direction on the gasket side of the plate	
				upper left	lower left	lower right	upper right		
				S1	S2	S3	S4		
				=<=	=>=	=>=	=<=		
		FRAME PLATE							
	1640917 01			O	O	O	O		
1	39505285 83	TS6 M2	A	O	O	O	O		
2	39505285 03	TS6 M2	B	U	--<---	U	O	O	Up
3	39505285 03	TS6 M2	A	O	O	U	--<---	U	Down
4	39505285 03	TS6 M2	B	U	--<---	U	O	O	Up
5	39505285 03	TS6 M2	A	O	O	U	--<---	U	Down
6	39505285 03	TS6 M2	B	U	--<---	U	O	O	Up
7	39505285 03	TS6 M2	A	O	O	U	--<---	U	Down
8	39505285 03	TS6 M2	B	U	--<---	U	O	O	Up
9	39505285 03	TS6 M2	A	O	O	U	--<---	U	Down
10	39505285 76	TS6 M2	B	--<---					Up
		PRESSURE PLATE		T1	T2	T3	T4		

Article No:	Quantity:
1640917 01	1
39505285 83	1
39505285 03	8
39505285 76	1



Alfa Laval, Plate Heat Exchanger Technical Specification

2015-08-12

Customer:	Viridia B2X, LLC	SU Order No:	85480-40
Model :	TS6-MFG	Serial No:	30116-93128
Customer PO:	148	Item No:	HE-2853

Fluid		Water-Steam	Water
Mass flow rate	lb/h	513.7	10000
Fluid Condensed/Vapourized	lb/h	513.7	0.000
Inlet temperature	°F	250.0	100.0
Outlet temperature (vapor/liquid)	°F	249.2/224.5	150.0
Operating pressure (In/Out)	psia	29.8/29.4	
Pressure drop (Perm/Calculate)	psi	14.5/0.424	14.5/0.494
Velocity Connection (In/Out)	ft/s	54.3/0.0658	1.22/1.23
Heat Exchanged	kBtu/h	498.5	
Mean Temperature Difference	°F	124.4	
Heat transfer area	ft ²	495.1	
O.H.T.C	Btu/ft ² ,h,°F	544.6	
Relative directions of fluids		Countercurrent	
Number of plates		10	
Number of passes		1	1
Extension capacity			4
Plate material/ Thickness		SA-240-316 / 0.60 mm	
Sealing material		EPDMP CLIP-ON	EPDMP CLIP-ON
Connection material		SA-240-316	SA-240-316
Connection diameter		3"	3"
Nozzle orientation		S4 -> S3	S1 <- S2
Pressure vessel code		ASME	
Flange rating		ASME 150#	
Design pressure	psig	150.0	150.0
Test pressure	psig	195.0	195.0
Design temperature	°F	300.0	300.0
Overall length x width x height	in	22 x 16 x 28	
Liquid volume	ft ³	0.07	0.09
Net Weight Empty / Operating	lbs	357 / 368	

Performance is conditioned on the accuracy of customers data and customers ability to supply equipment

Data, specifications, and otherkind of information of technological nature set out in this document and submitted by Alfa Laval to you (Proprietary Information) are intellectual proprietary rights of Alfa Laval. The Proprietary Information shall remain the exclusive property of Alfa Laval and shall only be used for the purpose of evaluating Alfa Laval's quotation. The Proprietary Information may not, without the written consent of Alfa Laval, be used or copied, reproduced, transmitted or communicated or disclosed in any other way to a third party.

FORM U-1 MANUFACTURER'S DATA REPORT FOR PRESSURE VESSELS

As Required by the Provisions of the ASME Code Rules, Section VIII, Division 1

1. Manufactured and certified by Alfa Laval Inc., 5400 International Trade Drive, Richmond, Virginia, 23231
(Name and address of Manufacturer)

2. Manufactured for Viridia B2X, LLC, 1319 HWY 182, Raceland, LA, 70394
(Name and address of Purchaser)

3. Location of installation Unknown
(Name and address)

4. Type Vertical Plate Heat Exchanger 30116-93128
(Horiz., vert., or sphere) (Tank, separator, jkt. vessel, heat exh., etc.) (Mfg's serial No.)

N/A 30116-93128.0 36557 2015
(CRN) (Drawing No.) (Nat'l. Bd. No.) (Year built)

5. ASME Code, Section VIII, Div. 1 2013/ N/A 2523, 2766 N/A
Edition and Addenda (date) Code Case No. Special Service per UG-120(d)

Items 6-11 incl. to be completed for single wall vessels, jackets of jacketed vessels, shell of heat exchangers, or chamber of multichamber vessels.

6. Shell (a) No. of course(s): 0 (b) Overall length: 0'

Course(s)			Material	Thickness		Long. Joint (Cat. A)			Circum. Joint (Cat. A, B, & C)			Heat Treatment	
No.	Diameter	Length	Spec./Grade or Type	Nom.	Corr.	Type	Full, Spot, None	Eff.	Type	Full, Spot, None	Eff.	Temp.	Time
	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>

7. Heads: (a) SA-516-70 (b) SA-516-70
(Mat'l Spec. No., Grade or Type) (H.T. - Time & Temp.) (Mat'l Spec. No., Grade or Type) (H.T. - Time & Temp.)

	Location (Top, Bottom, Ends)	Thickness		Radius		Elliptical Ratio	Conical Apex Angle	Hemispherical Radius	Flat Diameter	Side to Pressure		Category A		
		Min.	Corr.	Crown	Knuckle					Convex	Concave	Type	Full, Spot, None	Eff.
(a)	<u>Fixed</u>	<u>1.18"</u>	<u>0"</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>26" X 16"</u>			<u>N/A</u>	<u>N/A</u>	<u>N/A</u>
(b)	<u>Movable</u>	<u>1"</u>	<u>0"</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>26" X 16"</u>			<u>N/A</u>	<u>N/A</u>	<u>N/A</u>

If removable, bolts used (describe other fastening) SA193-B7 (6) 0.787" (M20 actual) BOLTS
(Mat'l, Spec. No., Grade, Size, No.)

8. Type of jacket N/A Jacket closure N/A
(Describe as ogee & weld, bar, etc.)

If bar, give dimensions N/A If bolted, describe or sketch.

9. MAWP 150 psi N/A at max. temp. 300 °F N/A Min. design metal temp. -20 °F at 150 psi
(internal) (external) (internal) (external)

10. Impact test NO (Impact Exemption UCS-66(a), (b), UHA-51, UNF-65, as applicable) at test temperature of N/A
(Indicate yes or no and the component(s) impact tested)

11. Hydro., pneu., or comb. test press. HYDRO at 195 psi Proof test N/A

Items 12 and 13 to be completed for tube sections.

12. Tubesheet: N/A N/A N/A N/A N/A
Stationary (Mat'l Spec. No.) Dia., (subject to press.) Nom. thk. Corr. Allow. Attachment (welded or bolted)

N/A N/A N/A N/A N/A
Floating (Mat'l Spec. No.) Dia. Nom. thk. Corr. Allow. Attachment

13. Tubes: N/A N/A N/A N/A N/A
Mat'l Spec. No., Grade or Type O. D. (Nom. thk.) Number Type (Straight or U)

Items 14-18 incl. to be completed for inner chambers of jacketed vessels or channels of heat exchangers.

14. Shell (a) No. of course(s): 0 (b) Overall length: N/A

Course(s)			Material	Thickness		Long. Joint (Cat. A)			Circum. Joint (Cat. A, B, & C)			Heat Treatment	
No.	Diameter	Length	Spec./Grade or Type	Nom.	Corr.	Type	Full, Spot, None	Eff.	Type	Full, Spot, None	Eff.	Temp.	Time
	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>

15. Heads: (a) N/A (b) N/A
(Mat'l Spec. No., Grade or Type) (H.T. - Time & Temp.) (Mat'l Spec. No., Grade or Type) (H.T. - Time & Temp.)

	Location (Top, Bottom, Ends)	Thickness		Radius		Elliptical Ratio	Conical Apex Angle	Hemispherical Radius	Flat Diameter	Side to Pressure		Category A		
		Min.	Corr.	Crown	Knuckle					Convex	Concave	Type	Full, Spot, None	Eff.
<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>	<u>N/A</u>			<u>N/A</u>	<u>N/A</u>	<u>N/A</u>

If removable, bolts used (describe other fastening) N/A
(Mat'l, Spec. No., Grade, Size, No.)

16. MAWP N/A N/A at max. temp. N/A N/A Min. design metal temp. N/A at N/A
 (internal) (external) (internal) (external)

17. Impact test N/A at test temperature of N/A
 (Indicate yes or no and the component(s) impact tested)

18. Hydro., pneu., or comb. test press. N/A Proof test N/A

19. Nozzles, inspection, and safety valve openings:

Purpose (Inlet, Outlet, Drain, etc.)	No.	Diameter or Size	Flange Type	Material		Nozzle Thickness		Reinforcement Material	How Attached		Location (Insp. Open.)
				Nozzle	Flange	Nom.	Corr.		Nozzle	Flange	
Outlet	2	3"	STUDS	SA193-B7		5/8"					
Inlet	2	3"	STUDS	SA193-B7		5/8"					

20. Supports: Skirt Lugs N/A Legs N/A Others FEET Attached BOLTED
 (Yes or no) (No.) (No.) (Describe) (Where and how)

21. Manufacturer's Partial Data Reports properly identified and signed by Commissioned Inspectors have been furnished for the following items of the report:
N/A
 (List the name of part, item number, mfg's. name and identifying number)

22. Remarks:
Actual Plates (10) SA-240-316 (ALLOY 316) 0.6 mm (13) Plates Maximum; Distance between Heads = 47 mm; Customer PO#: 148; Tag #: Desorbent Water Feed Heater HE-2853; Owner to supply Safety Valve/Noncorrosive Service Only;

CERTIFICATE OF SHOP COMPLIANCE

We certify that the statements made in this report are correct and that all details of design, material, construction, and workmanship of this vessel conform to the ASME Code for Pressure Vessels, Section VIII, Division 1. U Certificate of Authorization No. 25017 Expires July 5, 2016

Date 10/02/2015 Name Alfa Laval Inc. Signed [Signature]
 (Manufacturer) (Representative)

CERTIFICATE OF SHOP INSPECTION

I, the undersigned, holding a valid commission issued by The National Board of Boiler and Pressure Vessel Inspectors and/or the State or Province of VA and employed by OneCIS Insurance Company, of Lynn, MA have inspected the pressure vessel described in this Manufacturer's Data Report on October 2, 2015, and state that, to the best of my knowledge and belief, the Manufacturer has constructed this pressure vessel in accordance with ASME Code, Section VIII, Division 1. By signing this certificate neither the Inspector nor his employer makes any warranty, expressed or implied, concerning the pressure vessel described in this Manufacturer's Data Report. Furthermore, neither the Inspector nor his employer shall be liable in any manner for any personal injury or property damage or a loss of any kind arising from or connected with this inspection.

Date 10/14/2015 Signed [Signature] Commissions 10803A, VA951R
 (Authorized Inspector) (Nat'l Board incl. endorsements, State, Province and No.)

CERTIFICATE OF FIELD ASSEMBLY COMPLIANCE

We certify that the statements made in this report are correct and that the field assembly construction of all parts of this vessel conforms with the requirements of ASME Code, Section VIII, Division 1. U Certificate of Authorization No. Expires

Date Name Signed
 (Assembler) (Representative)

CERTIFICATE OF FIELD ASSEMBLY INSPECTION

I, the undersigned, holding a valid commission issued by The National Board of Boiler and Pressure Vessel Inspectors and/or the State or Province of and employed by of have compared the statements in this Manufacturer's Data Report with the described pressure vessel and state that parts referred to as data items , not included in the certificate of shop inspection, have been inspected by me and to the best of my knowledge and belief, the Manufacturer has constructed and assembled this pressure vessel in accordance with the ASME Code, Section VIII, Division 1. The described vessel was inspected and subjected to a hydrostatic test of psi. By signing this certificate neither the Inspector nor his employer makes any warranty, expressed or implied, concerning the pressure vessel described in this Manufacturer's Data Report. Furthermore, neither the Inspector nor his employer shall be liable in any manner for any personal injury or property damage or a loss of any kind arising from or connected with this inspection.

Date Signed Commissions
 (Authorized Inspector) (Nat'l Board incl. endorsements, State, Province and No.)



36557

Certified by

Alfa Laval - Richmond, VA

Single ratings apply to all chambers
Multiple ratings are listed as Hot/Cold

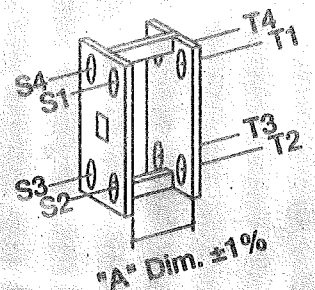
MAWP	150 PSIG AT 300 °F		
MDMT	-20 °F AT 150 PSIG		
S/N	30116-93128	Year	2015
Model	TS6 - MFG		
Area	7.4 SQ. FT.		
A-Dim	47mm w/10 0.6mm PLTS		
Order	1828203		

VIRDIA PO 148
 ITEM NO HE-2853
 DUTY 498,500 BTU/HR
 OHTC 544.6 BTU/FT²,H,F
 MAX NO OF 316 SS PLATES: 14

Customer Service

866-ALFALAVAL

WWW.ALFALAVAL.COM



October 7, 2015



Alfa Laval Inc.
5400 International Trade Drive
Richmond, VA 23232
USA
Tel: +1 804-222-5300
www.alfalaval.com

Reference:	Alfa Laval Order:	85480 / 1828203
	Serial Number:	30116-93128
	Customer Reference Number:	148
	Customer Item No:	HE-2853

CERTIFICATE OF CONFORMANCE

Alfa Laval hereby states that the methods and materials used to fabricate the plate heat exchanger(s) referenced above conform to the applicable requirements of Alfa Laval Richmond's ISO 9001-2008 Quality Management System requirements. Additionally, if an ASME code stamp has been applied to the equipment, we hereby state that it meets the applicable requirements of ASME Section VIII, Div.1.

Sincerely,

A handwritten signature in cursive script that reads "Michael J. Gunnoe".

Michael J. Gunnoe – Quality Technician, Quality Assurance
Alfa Laval Inc – USA, VA

MISSION STATEMENT: To optimize the performance of our customer's processes.
Time and time again.



FRAME & PRESSURE PLATES

NUCOR
PLATE MILL

P.O. Box 279
Winton, NC 27986
(252) 358-6700

Mill Test Report
Page 1



Issuing Date : 04/23/2013 B/L No. : 365310 Load No. : 358178 Our Order No. : 110890/6 Cust. Order No. : 2518100 OP
 Vehicle No: NOKL 725387 Sold To : ONEAL STEEL INC Ship To : ONEAL MANUFACTURING SERVICES
 Specification : 1.0000" x 96.000" x 240.000" P O BOX 98 DISTRICT 261
 ASTM A516 70-10/ASME SA516 70 SA516-485 PVQ 2011 Addenda BIRMINGHAM, AL 35201 STATION: PITTSBURGH 43RD STREET
 NACE MR0175 Annex 2.1.2, MR0103 Section 2.1.2 Compliant PITTSBURGH, PA 15202

Marking : 908958

Heat No	C	Mn	P	S	Si	Cu	Ni	Cr	Mo	Al(tot)	V	Nb	Ti	N	Ca	B	Sn	CEQ	PCM
3503059	0.19	1.02	0.020	0.001	0.21	0.12	0.04	0.08	0.01	0.034	0.004	0.003	0.002		0.0011	0.0003	0.006	0.39	0.26

Plate Serial No	Tensile Test					Charpy Impacts														
	Pieces	Tens	Dir.	Yield (ksi)	Tensile (ksi)	Elongation % in 2"	Elongation % in 8"	Dir.	1	(F) shear	2	(F) shear	3	(F) shear	Avg.	(F) shear	Size	Temp	Min Avg.	
3503059-02	13	42.47	T	44,200	70,200		24.0													

herbert.wathan@alfalaval.com
 Digitally signed by herbert.wathan@alfalaval.com
 DN: cn=herbert.wathan@alfalaval.com
 Date: 2015.10.06 09:05:34 -04'00'

Manufactured to fully killed fine grain practice by Electric Arc Furnace. Welding or weld repair was not performed on this material. Mercury has not been used in the direct manufacturing of this material. Produced as continuous cast discrete plate as-rolled, unless otherwise noted in Specification.
 Yield by 0.5EUL method unless otherwise specified. $C_{eq} = C + (Mn/5) + ((Cr+Mo+V)/5) + ((Cu+Ni)/15)$
 $P_{cm} = C + (Si/30) + (Mn/20) + (Cu/20) + (Ni/60) + (Cr/20) + (Mo/15) + (V/10) + 5B$
 Melted and manufactured in the USA, ISO 9001:2008 certified (#0088083) by SRI Quality System Registrar (#0985-09), PED 97/23/EC 7/2 Annex 1, Para. 4.3 Compliant.
 DIN 50049 3.1.B/EN 10204 3.1.B(2004), DIN EN 10204 3.1(2008) compliant. For ABS grades only, Quality Assurance certificate 09-MMPQA-546
 We hereby certify that the contents of this report are accurate and correct. All test results and operations performed by the material manufacturer are in compliance with the applicable specifications, including customer specifications.
 T. A. Depratis, Metallurgist
 04/25/2013 6:31:24 AM

P.O.Box 279
Winton, NC 27986
(252) 356-3700

Issuing Date : 05/27/2015 B/L No. : 416925 Load No. : 422523 Our Order No. : 127883/1 Cust. Order No. : 4500684520
 Vehicle No: 10451 Sold To: RYERSON PROCUREMENT CORP Ship To: JOSEPH T RYERSON AND SON INC -
 Specification: 1.181" x 96.000" x 240.000" PO BOX 91601 FAIRLESS HILLS
 ASTM A516 70-10/ASME SA516 70 SA516-485 PVQ 2011 Addenda, 2013 LUBBOCK, TX 79490 20 STEEL RD S
 NACE MR0175 Annex 2.1.2, MR0103 Section 2.1.2 Compliant MORRISVILLE, PA 19067

Marking : 300041821

Heat No	C	Mn	P	S	Si	Cu	Ni	Cr	Mo	Al(tot)	V	Nb	Ti	N	Ca	B	Sn	Ceq	Pcm
5502764	0.20	1.06	0.013	0.003	0.21	0.21	0.08	0.11	0.02	0.027	0.004	0.001	0.005		0.0027	0.0003	0.009	0.42	0.27

Tensile Test

Pieces	Tons	Dir.	Yield (psi)	Tensile (psi)	Elongation % in 2"	Elongation % in 8"
1	3.85	T	43,600	74,200	18.0	18.0

Charpy Impacts


Dir.	1	2	3	Ave.	(%) shear	Size	Temp

Digitally signed by
 herbert.wathan@alfalaval.com
 DN:
 cn=herbert.wathan@alfalaval.com
 Date: 2015.07.13 14:56:52 -04'00'

Manufactured to fully killed fine grain practice by Electric Arc Furnace. Welding or weld repair was not performed on this material. We hereby certify that the contents of this report are accurate and correct. All test results and Mercury has not been used in the direct manufacturing of this material. Produced as continuous cast discrete plate as-rolled, unless otherwise noted in Specification. For Mexico shipments: nhc-sales.mx@nucor.com operations performed by the material manufacturer are in compliance with the applicable specifications, including customer specifications.
 Yield by 0.5EUL method unless otherwise specified. Ceq = C+(Mn/6)+(Cr+Mo+V)/5+(Cu+Ni)/15
 Pcm = C+(Si/30)+(Mn/20)+(Cu/20)+(Ni/60)+(Cr/20)+(Mo/15)+(V/10)+58
 Melted and Manufactured in the USA. ISO 9001:2008 certified (#010940) by SRI Quality System Registrar (#0985-09). PED 97/23/EC 7/2 Annex 1, Para. 4.3 Compliant.
 DIN 50049 3.1.B/EN 10204 3.1B(2004), DIN EN 10204 3.1(2005) compliant. For ABS grades only, Quality Assurance certificate 14-MMPQA-723
 T. A. Depreis, Metallurgist
 5/27/2015 11:25:57 AM



CHANNEL PLATES

 <p>Aperam - Stainless France Aperam Gueugnon 71130 Gueugnon FRANCE</p>		MILL CERTIFICATE BS EN 10204/3.1 CERTIFICAT DE RECEPTION NF EN 10204/3.1 ABNAHMEPRUEFZEUGNIS DIN EN 10204/3.1								N-Nr-N 15G0339755-01 V01																							
		ISO 9001 V2008 - ISO TS 16949 V2009 - ISO 14001 V 2004																															
Manufacturer's works order number N° de la commande usine productrice Werksauftragsnummer 80251884 /01-60770/1		Purchaser and/or consignee Client et/ou destinataire Besteller und/oder Empfaenger Alfa Laval Lund AB Rudeboksvägen 1 221 00 LUND SUEDE		Purchaser's order number N° de commande client Kundenbestellnummer OMCP-133699																													
Product - Produit - Erzeugnis COLD-ROLLED COIL BOBINE LAMINEE A FROID KALTGEWALZTES BAND		Customer article number N.article client Artikelnummer des Kunden																															
Steel designation Désignation de l'acier Stahlbezeichnung ASTM A 240 / 14 - TYPE 316L - TYPE 316 ASME SA 240 / 13 - TYPE 316L - TYPE 316 EN 10028-7 / 08 - 1.4404 - 1.4401 EN 10088-2 / 14 - 1.4404 - 1.4401		Finish Présentation Ausführung 2B 2B 2B 2B		Steelmaking process Mode d'élaboration de l'acier - Stahlherstellungsverfahren Prod.proces: Electric arc furnace - VOD/AOD - Continuous casting Proc.fabric.: Four à arc - VOD/AOD - Coulée continue Fertigungsablauf: Elektro-Ofen - VOD/AOD - Stranggussanlage				Product delivery condition Etat de livraison du produit - Lieferzustand Solution treated Hypertrempe : 1040-1110 C Loesungsgegl+abgeschreckt																									
		Any supplementary requirements Prescriptions supplémentaires - Zusätzliche Anforderungen AL1112359-63/5-AL1112349-		Forced Air Air forcé Gebläse Luft																													
NACE MR 0103 / ATTESTATION PED 97/23/EC PAR TUV SUD / CORROSIONTEST:ASTM A 262-E :OK / NACE MR 0175/ISO 15156-1 / ISO 15156-3 //																																	
Identification of the product Identification du produit - Identifizierung des Erzeugnisses MELTED IN BELGIUM, MADE IN FRANCE		Dimensions Dimensions - Abmessungen Thickness B09 Epaisseur - Staerke 0,600 mm				Width B10 Largeur - Breite 333,00 mm		Length B11 Longueur - Laenge		Number of pieces B08 Nb de pièces - Stueckzahl 1																							
Coil n. N. Bobine - Band Nr. 731306		Heat n. N.Coulée - Schmelz Nr. 507025								Net weight B13 Poids net - netto Gewicht 1314 KGS																							
CHEMICAL ANALYSIS - ANALYSE CHIMIQUE - CHEMISCHE ZUSAMMENSETZUNG																																	
		C		Si		Mn		Ni		Cr		Mo		Ti		N		S		P													
Required -Exigé % mini Anforderung. %maxi		0,030		0,75		2,00		10,00 13,00		16,50 18,00		2,000 2,500				0,100		0,0150		0,045													
Cast Analysis Analyse coulée Analyse Schmelze		0,026		0,37		1,25		10,03		16,56		2,020				0,040		0,0018		0,031													
		C71		C72		C73		C74		C75		C76		C77		C78		C79		C80		C81		C82		C83		C84		C85		C86	
Positive material identification carried out : OK Tests de vérification de la conformité de la nuance fournie : OK Verwechslungsprüfung wurde durchgeführt : OK																																	
Location (1)		MECHANICAL PROPERTIES - PROPRIETES MECANIQUES - MECHANISCHE WERTE																															
		Room temperature - Température ambiante - Raumtemperatur										Test temperature (°C) :																					
Direction (2)		Yield or proof strength Limite d'élasticité Dehngrenze MPa		Tensile Strength Résistance à la traction Zugfestigkeit MPa		Elongation after fracture Allongement après rupt. Bruchdehnung %		Hardness Dureté Haerte		Yield or proof strength Limite d'élasticité Dehngrenze MPa		Tensile str. Résist. MPa Zugfestigkeit		Elongation % Allongement. Bruchdehnung																			
		Required Exigé Anforderung		Rp0.2% Rp1%		Rm		80mm 50mm		HB C30		Rp0.2% Rp1%		Rm																			
		mini maxi		240 270		530 680		45 45		130 180																							
1 T		Obtained Obtenu		285 330		592		62 62		154																							
2 T		Ergebnisse		C11 C14		C12		C13 C15		C31 155		C16 C17		C18 C19																			
Impact strength test Essai de résilience Kerbschlagzähigkeitstest				Corrosion test Test de corrosion Korrosionstest		GRAIN ASTM E 112																											
		C40 t(°c)		C44				8,4 C50 C51		C52 C53 C54 C55				C05																			
		C42						Internal cleanliness:		A: B: C: D:				C57																			
Location of the sample (1) Emplacement de l'échantillon Lage des Probenabschnittes 1. Front - Début - Anfang 2. Back - Fin - Ende 3. Middle - Milieu - Mitte		The delivery is in accordance with the order La fourniture est conforme aux exigences de la commande Die lieferung entspricht den Bestellbedingungen		Packing list Avis d'expédition Lieferscheinnummer 150311G01724-101132		Organisation inspection Organisme et/ou service contrôle Ueberwachungsabteilung		Customer Quality 11/03/2015 Romain GERARD		The inspector Le responsable Der Werkssachverstaendige																							
Direction of the test pieces (2) Orientation des éprouvettes Probenrichtung T. Transverse - Travers - Quer L. Longitudinal - Long - Laengs																																	



TIGHTENING BOLTS/NUTS



Certificat d'essais / Test Certificate NFEN 10204.3.1

NR 15026/26 - 1260/99 - 61698

ALFA LAVAL INC

5400 INTERNATIONAL TRADE DRIVE
 RICHMOND, VIRGINIA 23231 U.S.A.

VOTRE/YOUR REF N° US-158897

Poste/item 1 VIS M20X200/150 ENISO 898-1 CL8.8
 Qte/Qty 400 SA193/B7 ADW7
 COAT : ZN
 Plan 32324-1402-01 REV.9

- VIS M20X200/150
 Coulée : B09102

Normes/Standard : ASME SA193 GRADE B7- EN ISO 898-1 CL 8.8 ADW7

ANALYSE/HEAT ANALYSIS

C	Mn	Si	S	P	Cr	Mo	
0.41	0.83	0.22	0.008	0.01	0.86	0.16	

CARACTERISTIQUES MECANIQUES/MECHANICAL TEST

TEST n°85326	Rm 300°C MPa	Rp0.2% 300° MPa	A% 5d 300°C	Z% 300°C			
	899	656	26.4	71			
TEST n°85327	Rm MPa	Rp0.2% MPa	A% 4D	A% 5d	Z%	REV. T° C	HB
	947	861	21.5	18.2	63	640	287
TEST n°85329	Rm MPa	Rp0.2% MPa	A% 4D	A% 5d	Z%	REV. T° C	HB
	961	878	21.3	18.4	61	640	317
TEST n°85330	KV +20°C Joules						
	129/129/122						
TEST n°85331	Rm MPa	Rp0.2% MPa	A% 4D	A% 5d	Z%	REV. T° C	HB
	950	864	20.6	18.1	62	640	287
TEST n°85332	KV +20°C Joules						
	123/120/120						
TEST n°85333	Rm MPa	Rp0.2% MPa	A% 4D	A% 5d	Z%	REV. T° C	HB
	965	879	21.3	18.7	61	640	317

Goods are in conformity to: EN ISO 898-1 CLASSE 8.8 ADW7-ASME SA193 GRADE B7 EDITION 2010 + ADDENDAS 2011
 Material in conformity To PED 97/23/EC
 Delivery state : Quenched and Tempered
 Superficial hardness : HV 0.3 : Conform
 Decarburatation Test : Conform
 Proof load Test : Conform
 Proof load Test with wedge : Conform
 Visual and dimensional control : conform
 Macroetch Examination :Conform
 Marks on tightbolt : BC - B7 - 8.8
 V X PS <= 5000
 MIT ZUSTIMMUNG DES TÜV RHEINLAND WE 208 VOM 15.01.85 UND VERZICHT AUF GEGENZEICHNUNG VOM 15.10.99

**Certificat d'essais / Test Certificate NFEN 10204.3.1**

NR 15026/26 - 1260/99 - 61698

ALFA LAVAL INC5400 INTERNATIONAL TRADE DRIVE
RICHMOND, VIRGINIA 23231 U.S.A.VOTRE/YOUR REF N° US-158897

We the undersigned, BECK CRESPEL, hereby certify that the supplied products are fully in accordance with the requirements of the order.

This test certificate applies only the items, designation and quantities as described above.



Alfa Laval Inc.
5400 International Trade Dr.
Richmond, VA 23231

Date: 1/6/12

Rev.: 9

Title: Hydrostatic Test Certificate

Doc. No.: Form 25002

Page 1 of 1

CUSTOMER	Viridia B2X, LLC
P.O. NO.	148
ALFA LAVAL NO.	85480 / 1828203
SERIAL NO.	30116-93128
ITEM NO. or MODEL TYPE	HE-2853

ASME Hydrostatic Test No.	Revision No.
WI 25001	9

	Test Pressure	Holding Time	Test Gauges	Gauge Due Date	A.I. Witness or Test Operator
Single	195 PSIG / 150 PSIG	2 min / 10 min	620	3/9/2016	Herb Wathan (A.I.)
Single	195 PSIG / 150 PSIG	2 min / 10 min	620	3/9/2016	Herb Wathan (A.I.)

C.I. Hydrostatic Test No.	Revision No.
N/A	N/A

	Test Pressure	Holding Time	Test Gauges	Gauge Due Date	Customer Witness
Single	N/A	N/A	N/A	N/A	N/A
Single	N/A	N/A	N/A	N/A	N/A

This hydrostatic pressure test was performed successfully with no visible internal or external leakage from the heat exchanger. Testing was witnessed as noted above.

APPROVED BY: Michael J. Gunnoe (Quality Technician)

DATE OF ASME CODE TEST: 10/2/2015

DATE OF CUSTOMER WITNESS: Waived per Jim Biasca 9/22/2015

Prepared By: Jason Gunnoe	Approved By: Mike Pischke
Revision Description: Rev 7: Add sign off for CI inspection Rev 8: added test operator to the selections for non-code/UM PHEs. Rev.9: Added gauge number field & Gauge date field.	

Customer: Virdia B2X, LLC
 Model : TS6-MFG
 Customer PO: 148

SU Order No: 85480-40
 Serial No: 30116-93128
 Item No: HE-2853

Design Verification of Plate Heat Exchangers (PHE) exposed to External Loads

ASME Section III, Division 1
 Edition and Addenda according to doc. PPD 0406-07

Order number 85480-40
 Document registration number 30116-93128
 Revision 0

0	External Load Analysis: Seismic Accelerations per ASCE 7-10		
Rev. no.	Description of revision		
Prepared	Date	Signature	
Randy Neagle	8/21/2015	Randy Neagle	
Approved	Date	Signature	
Dean Middleton	8/21/2015	Dean Middleton	



Calculation of External Loads



Seismic Loads (ASCE 7-10)

Order Number: 85480
 Serial Numbers: 30116-93125
 30116-93126
 30116-93128
 30116-93129
 30116-93130
 30116-93131

Site location:	Raceland, LA		
Risk Category:	II		Based on Table 1.5-1 (Assumed)
Importance Factor:	$I := 1.00$		Based on Description in Section 11.5 and Table 1.5-2
Site Classification:	E		Based on Description in Section 11.4.2 & Chapter 20 and Supplied Inputs
Mapped spectral acceleration at short periods as determined in Section 11.4:	$S_S := 0.125$		Based on Customer Inputs and Supplied Site Information
Mapped spectral acceleration at a period of 1-second as determined in Section 11.4:	$S_1 := 0.073$		Based on Customer Inputs and Supplied Site Information
Site coefficient defined in Table 11.4-1:	$F_a := 2.5$		Based on Customer Inputs and Supplied Site Information
Site coefficient defined in Table 11.4-2:	$F_v := 3.5$		Based on Customer Inputs and Supplied Site Information
Max. considered earthquake spectral response acceleration for short periods:	$S_{MS} := F_a \cdot S_S$	$S_{MS} = 0.313$	(ASCE Eq. 11.4-1)
Max. considered earthquake spectral response acceleration at 1 second:	$S_{M1} := F_v \cdot S_1$	$S_{M1} = 0.256$	(ASCE Eq. 11.4-2)
Design spectral response acceleration at short periods	$S_{DS} := \frac{2 \cdot S_{MS}}{3}$	$S_{DS} = 0.208$	(ASCE Eq. 11.4-3)
Design spectral response acceleration at 1 second:	$S_{D1} := \frac{2 \cdot S_{M1}}{3}$	$S_{D1} = 0.17$	(ASCE Eq. 11.4-4)

Component Amplification Factor	$a_p := 1.0$	Based on Tables 13.5-1 and 13.6-1
Component Response Modification Factor	$R_p := 2.5$	Based on Tables 13.5-1 and 13.6-1
Height in Structure Point of Attachment with Respect to the Base	$z := 0$	Heat exchanger assumed to be mounted at base.
Height of Structure	$h := 0$	Heat exchanger assumed to be mounted at base.
Seismic Design Acceleration (Horizontal):	$a_{\text{horizontal}} := \frac{(0.4 \cdot a_p \cdot S_{DS})}{\left(\frac{R_p}{I}\right)} \left[1 + 2 \cdot \left(\frac{z}{h}\right) \right]$ $a_{\text{horizontal}} = 0.033$	(ASCE Eq. 13.3-1)
Horizontal acceleration is not required to be taken as greater than:	$a_{\text{horizontal}} := 1.6 \cdot S_{DS} \cdot I$ $a_{\text{horizontal}} = 0.333$	(ASCE Eq. 13.3-2)
Horizontal acceleration shall not be taken as less than:	$a_{\text{horizontal}} := 0.3 \cdot S_{DS} \cdot I$ $a_{\text{horizontal}} = 0.063$	(ASCE Eq. 13.3-3)
Seismic Design Acceleration (Vertical):	$a_{\text{vertical}} := (0.2 \cdot S_{DS})$ $a_{\text{vertical}} = 0.042$	Based on Section 13.3


Design Maps Detailed Report

2012 International Building Code (38.54484°N, 82.76962°W)

Site Class E – “Soft Clay Soil”, Risk Category I/II/III

Section 1613.3.1 — Mapped acceleration parameters

Note: Ground motion values provided below are for the direction of maximum horizontal spectral response acceleration. They have been converted from corresponding geometric mean ground motions computed by the USGS by applying factors of 1.1 (to obtain S_s) and 1.3 (to obtain S_1). Maps in the 2012 International Building Code are provided for Site Class B. Adjustments for other Site Classes are made, as needed, in Section 1613.3.3.

From [Figure 1613.3.1\(1\)](#) ^[1] $S_s = 0.152 \text{ g}$ **From [Figure 1613.3.1\(2\)](#) ^[2]** $S_1 = 0.073 \text{ g}$ **Section 1613.3.2 — Site class definitions**

The authority having jurisdiction (not the USGS), site-specific geotechnical data, and/or the default has classified the site as Site Class E, based on the site soil properties in accordance with Section 1613.

2010 ASCE-7 Standard – Table 20.3-1
SITE CLASS DEFINITIONS

Site Class	\bar{v}_s	\bar{N} or \bar{N}_{ch}	\bar{s}_u
A. Hard Rock	>5,000 ft/s	N/A	N/A
B. Rock	2,500 to 5,000 ft/s	N/A	N/A
C. Very dense soil and soft rock	1,200 to 2,500 ft/s	>50	>2,000 psf
D. Stiff Soil	600 to 1,200 ft/s	15 to 50	1,000 to 2,000 psf
E. Soft clay soil	<600 ft/s	<15	<1,000 psf
Any profile with more than 10 ft of soil having the characteristics:			
<ul style="list-style-type: none"> • Plasticity index $PI > 20$, • Moisture content $w \geq 40\%$, and • Undrained shear strength $\bar{s}_u < 500 \text{ psf}$ 			
F. Soils requiring site response analysis in accordance with Section 21.1	See Section 20.3.1		

For SI: 1ft/s = 0.3048 m/s 1lb/ft² = 0.0479 kN/m²

Section 1613.3.3 — Site coefficients and adjusted maximum considered earthquake spectral response acceleration parameters

TABLE 1613.3.3(1)
VALUES OF SITE COEFFICIENT F_a

Site Class	Mapped Spectral Response Acceleration at Short Period				
	$S_s \leq 0.25$	$S_s = 0.50$	$S_s = 0.75$	$S_s = 1.00$	$S_s \geq 1.25$
A	0.8	0.8	0.8	0.8	0.8
B	1.0	1.0	1.0	1.0	1.0
C	1.2	1.2	1.1	1.0	1.0
D	1.6	1.4	1.2	1.1	1.0
E	2.5	1.7	1.2	0.9	0.9
F	See Section 11.4.7 of ASCE 7				

Note: Use straight-line interpolation for intermediate values of S_s

For Site Class = E and $S_s = 0.152$ g, $F_a = 2.500$

TABLE 1613.3.3(2)
VALUES OF SITE COEFFICIENT F_v

Site Class	Mapped Spectral Response Acceleration at 1-s Period				
	$S_1 \leq 0.10$	$S_1 = 0.20$	$S_1 = 0.30$	$S_1 = 0.40$	$S_1 \geq 0.50$
A	0.8	0.8	0.8	0.8	0.8
B	1.0	1.0	1.0	1.0	1.0
C	1.7	1.6	1.5	1.4	1.3
D	2.4	2.0	1.8	1.6	1.5
E	3.5	3.2	2.8	2.4	2.4
F	See Section 11.4.7 of ASCE 7				

Note: Use straight-line interpolation for intermediate values of S_1

For Site Class = E and $S_1 = 0.073$ g, $F_v = 3.500$

Equation (16-37): $S_{MS} = F_a S_s = 2.500 \times 0.152 = 0.380 \text{ g}$

Equation (16-38): $S_{M1} = F_v S_1 = 3.500 \times 0.073 = 0.256 \text{ g}$

Section 1613.3.4 — Design spectral response acceleration parameters

Equation (16-39): $S_{DS} = \frac{2}{3} S_{MS} = \frac{2}{3} \times 0.380 = 0.253 \text{ g}$

Equation (16-40): $S_{D1} = \frac{2}{3} S_{M1} = \frac{2}{3} \times 0.256 = 0.171 \text{ g}$

Section 1613.3.5 — Determination of seismic design category

TABLE 1613.3.5(1)

SEISMIC DESIGN CATEGORY BASED ON SHORT-PERIOD (0.2 second) RESPONSE ACCELERATION

VALUE OF S_{DS}	RISK CATEGORY		
	I or II	III	IV
$S_{DS} < 0.167g$	A	A	A
$0.167g \leq S_{DS} < 0.33g$	B	B	C
$0.33g \leq S_{DS} < 0.50g$	C	C	D
$0.50g \leq S_{DS}$	D	D	D

For Risk Category = I and $S_{DS} = 0.253 g$, Seismic Design Category = B

TABLE 1613.3.5(2)

SEISMIC DESIGN CATEGORY BASED ON 1-SECOND PERIOD RESPONSE ACCELERATION

VALUE OF S_{D1}	RISK CATEGORY		
	I or II	III	IV
$S_{D1} < 0.067g$	A	A	A
$0.067g \leq S_{D1} < 0.133g$	B	B	C
$0.133g \leq S_{D1} < 0.20g$	C	C	D
$0.20g \leq S_{D1}$	D	D	D

For Risk Category = I and $S_{D1} = 0.171 g$, Seismic Design Category = C

Note: When S_1 is greater than or equal to 0.75g, the Seismic Design Category is **E** for buildings in Risk Categories I, II, and III, and **F** for those in Risk Category IV, irrespective of the above.

Seismic Design Category \equiv "the more severe design category in accordance with Table 1613.3.5(1) or 1613.3.5(2)" = C

Note: See Section 1613.3.5.1 for alternative approaches to calculating Seismic Design Category.

References

1. *Figure 1613.3.1(1)*: [http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/IBC-2012-Fig1613p3p1\(1\).pdf](http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/IBC-2012-Fig1613p3p1(1).pdf)
2. *Figure 1613.3.1(2)*: [http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/IBC-2012-Fig1613p3p1\(2\).pdf](http://earthquake.usgs.gov/hazards/designmaps/downloads/pdfs/IBC-2012-Fig1613p3p1(2).pdf)

GLOBAL DEFINITIONS

SCHEMATIC PICTURES

Plate heat exchanger parts

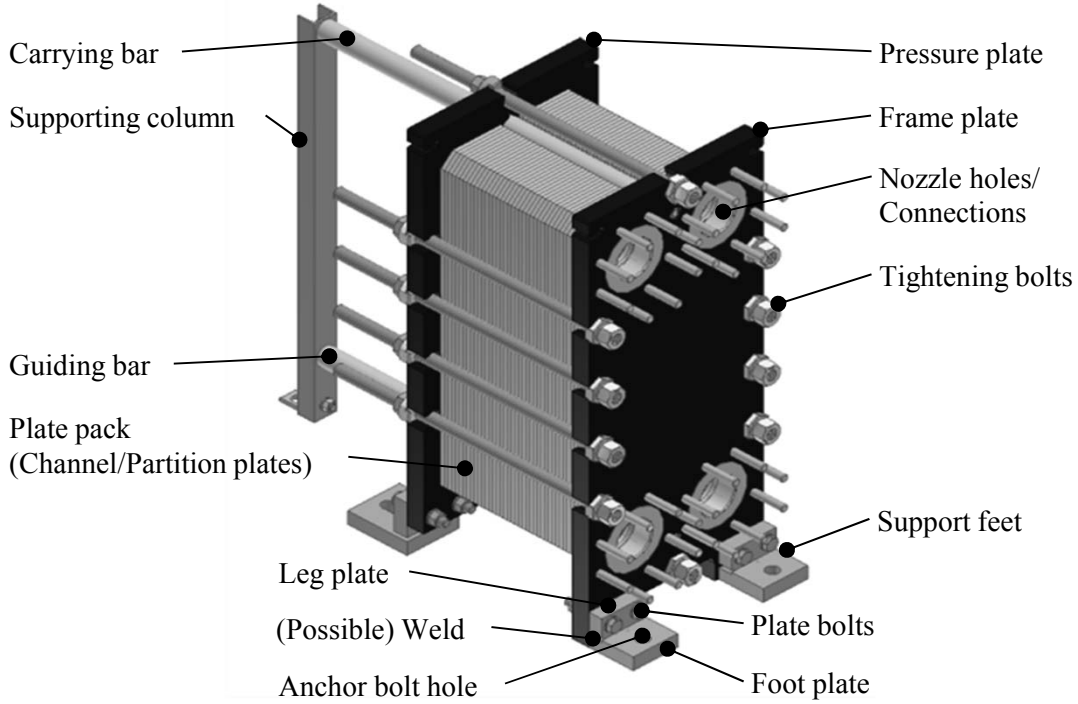
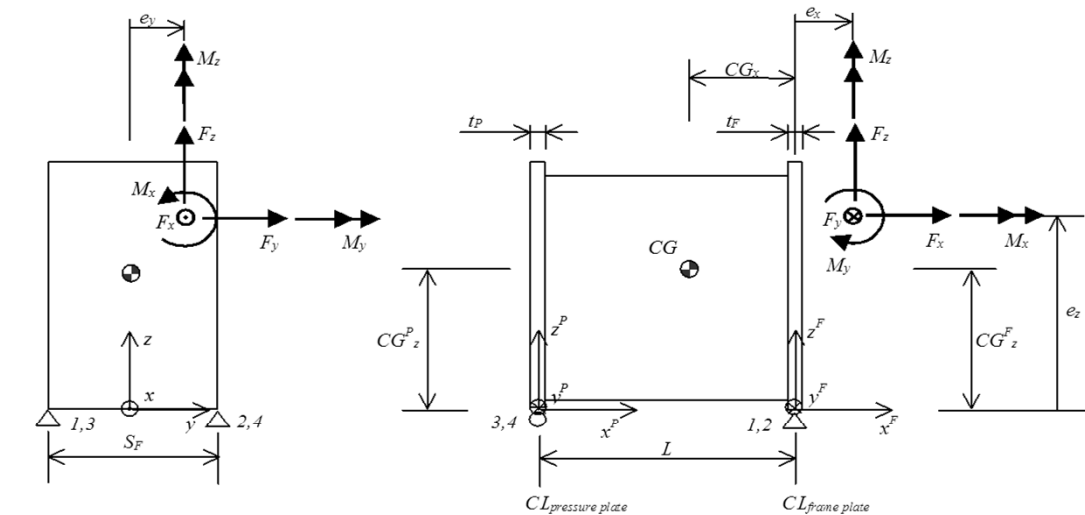


Plate heat exchanger model



CG=Centre of gravity

CL=Centre line (in x-direction)

The heat exchanger is assumed symmetrical about the xz-midplane with respect to geometry and weight.

To distinguish between parameters relating to the frame- or pressure- plate, the superscript F and P are used respectively.

The origins $(0,0,0)^F$ and $(0,0,0)^P$ are located in the yz-center plane of the frame/pressure plate, in the middle of the frame/pressure plate in the y-direction and in the z-coordinate of $CG_{plate\ bolts}$ (=the center of gravity of the plate bolt group).

COMPLEMENTARY DATA

GEOMETRY

Dimensions

Distance in the x-direction between the center lines of the frame and pressure plates

$$L = 74,5 \text{ [mm]}$$

Frame plate width, height, thickness and distance between the feet/foot pairs

$$B_F = 400 \text{ [mm]} \quad t_F = 30 \text{ [mm]}$$

$$H_F = 652 \text{ [mm]} \quad S_F = 254 \text{ [mm]}$$

Pressure plate width, height, thickness and distance between the feet/foot pairs

$$B_P = 400 \text{ [mm]} \quad t_P = 25 \text{ [mm]}$$

$$H_P = 652 \text{ [mm]} \quad S_P = 254 \text{ [mm]}$$

Height from frame and pressure plate bottom edge respectively to the lower tightening bolts category I

$$h_{tbF} = 225 \text{ [mm]} \quad h_{tbP} = 225 \text{ [mm]}$$

Height from frame and pressure plate bottom edge respectively to the lowest plate bolt holes

$$h_{Fpb} = 20 \text{ [mm]} \quad h_{Ppb} = 20 \text{ [mm]}$$

Center of gravity

Distance in the -x- and +z- directions from $(0,0,0)^F$ to center of gravity

For the PHE

$$CG_{x0} = +43 \text{ [mm]} \quad CG_{z0}^F = 378 \text{ [mm]}$$

For the additional mass

$$CG_{xa} = \text{ [mm]} \quad CG_{za} = \text{ [mm]}$$

MASS

Mean density of the heat transfer media

$$\rho = 1000 \text{ [kg/m}^3\text{]}$$

Mass of PHE with content

$$m_s = 170 \text{ [kg]}$$

Additional mass

$$m_a = 0 \text{ [kg]}$$

LOAD SPECIFICATION - Load analysis

Design/Operating temperature

$$T_G = +149 \text{ [}^\circ\text{C]}$$

Code Class

3

Service Level

B

Each load component is categorized as (i) static with known sign, (ii) static with unknown sign or dynamic and correlated (linearly summed) and (iii) dynamic and uncorrelated (SRSS-summed). Loads of types (i), (ii) and (iii) are specified in the 1:st, 2:nd and 3:rd row respectively. The e_z -values are normally conservatively simplified to relate to the bottom edge of the frame/pressure plate

NOZZLE LOADS AND POINTS OF ACTION

	S1	S2	S3	S4	S5	T1	T2	T3	T4	T5
F_x	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0 [N]
	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0
F_y	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0 [N]
	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0
F_z	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0 [N]
	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0
M_x	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0 [Nm]
	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0
M_y	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0 [Nm]
	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0
M_z	0	0	0	0	0	0	0	0	0	0
	0	0	0	0	0	0	0	0	0	0 [Nm]
	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0	± 0
e_x	0	0	0	0	0	0	0	0	0	0 [mm]
e_y	0	0	0	0	0	0	0	0	0	0 [mm]
e_z	0	0	0	0	0	0	0	0	0	0 [mm]

INERTIA LOADS

$$g \quad 9,81 \text{ [m/s}^2\text{]}$$

Gravity load

$$g_x \quad 0 \text{ [g]} \quad g_y \quad 0 \text{ [g]} \quad g_z \quad -1 \text{ [g]}$$

Acceleration loads

$$a_x \quad 0 \text{ [g]} \quad a_y \quad 0 \text{ [g]} \quad a_z \quad 0 \text{ [g]}$$

$$a_x \quad 0 \text{ [g]} \quad a_y \quad 0 \text{ [g]} \quad a_z \quad 0 \text{ [g]}$$

$$\pm 0,063 \text{ [g]} \quad \pm 0,063 \text{ [g]} \quad \pm 0,042 \text{ [g]}$$

EXTERNAL PRESSURE LOADS

Wind

$$v_x \quad 0 \text{ [m/s]} \quad v_y \quad 0 \text{ [m/s]} \quad v_z \quad 0 \text{ [m/s]}$$

$$v_x \quad 0 \text{ [m/s]} \quad v_y \quad 0 \text{ [m/s]} \quad v_z \quad 0 \text{ [m/s]}$$

$$\pm 0 \text{ [m/s]} \quad \pm 0 \text{ [m/s]} \quad \pm 0 \text{ [m/s]}$$

Pressure difference

$$q_x \quad 0 \text{ [bar]} \quad q_y \quad 0 \text{ [bar]} \quad q_z \quad 0 \text{ [bar]}$$

$$q_x \quad 0 \text{ [bar]} \quad q_y \quad 0 \text{ [bar]} \quad q_z \quad 0 \text{ [bar]}$$

$$\pm 0 \text{ [bar]} \quad \pm 0 \text{ [bar]} \quad \pm 0 \text{ [bar]}$$

SINGLE SUPPORT FOOT C ON THE FRAME PLATE

INPUT

Dimensions

PLATE BOLTS

Nominal diameter and thread pitch

$d_{pb} =$	16 [mm]	Type of bolt <input checked="" type="radio"/> Metric <input type="radio"/> Inch
$z_{pb} =$	2 [mm]	

Tensile stress, root, nominal and shear stress areas

$$A_{spb} = \begin{cases} \frac{\pi}{4} \cdot (d_{pb} - 0.9382 \cdot z_{pb})^2 & \text{metric screws} \\ \frac{\pi}{4} \cdot (d_{pb} - 0.9743 \cdot z_{pb})^2 & \text{unified inch screws} \end{cases} = 156,668 \text{ [mm}^2\text{]}$$

$$A_{rpb} = \frac{\pi}{4} \cdot (d_{pb} - 1.2990 \cdot z_{pb})^2 = 141,068 \text{ [mm}^2\text{]}$$

$$A_{npb} = \frac{\pi}{4} \cdot d_{pb}^2 = 201,062 \text{ [mm}^2\text{]}$$

Type of shear planes

<input type="radio"/> Threads not excluded from shear planes	$\Rightarrow A_{vpb} = A_{rpb}$	$\Rightarrow A_{vpb} = 201,062 \text{ [mm}^2\text{]}$
<input checked="" type="radio"/> Threads excluded from shear planes	$\Rightarrow A_{vpb} = A_{npb}$	

LEG PLATE

$$h_l = 57 \text{ [mm]} \quad d_{bhl} = 18 \text{ [mm]} \quad h_{pb} = 54,5 \text{ [mm]}$$

$$b_l = 90 \text{ [mm]} \quad s_{ypb} = 50 \text{ [mm]}$$

$$t_l = 35 \text{ [mm]} \quad h_{Fpb}, h_{Ppb} = 20 \text{ [mm]}$$

$$h_{ie} = h_l - h_{pb} + \frac{t_f}{2} = 20 \text{ [mm]}$$

$$h_e = h_{pb} - (h_{Fpb} \text{ or } h_{Ppb}) = 34,5 \text{ [mm]}$$

$$h_c = h_{pb} = 54,5 \text{ [mm]}$$

$$b_e = \frac{b_l - s_{ypb}}{2} = 20 \text{ [mm]}$$

Distance from the center of the plate bolt holes to the nearest leg plate edge

$$L_{bhl1} = L_{bhl2} = \min[b_e, h_{ie}, (h_{pb} - \frac{t_f}{2})] = 20 \text{ [mm]}$$

Shortest distance between the centers of the plate bolt holes

$$L_{sbhl} = s_{ypb} = 50 \text{ [mm]}$$

FOOT PLATE

$$\begin{array}{llll} l_{fa} = & 67,5 \text{ [mm]} & N_{ab} = & 1 \text{ [-]} & L_{bhf} = & 35 \text{ [mm]} \\ l_{fe} = & 35 \text{ [mm]} & d_{bhf} = & 32 \text{ [mm]} & L_{sbhf} = & \text{ [mm]} \\ b_f = & 90 \text{ [mm]} & & & & \\ t_f = & 35 \text{ [mm]} & & & & \end{array}$$

ANCHOR BOLTS

Nominal diameter and thread pitch

$$\begin{array}{ll} d_{ab} = & 24 \text{ [mm]} \\ z_{ab} = & 3 \text{ [mm]} \end{array}$$

Type of bolt
 Metric
 Inch

Tensile stress and root areas

$$A_{sab} = \begin{cases} \frac{\pi}{4} \cdot (d_{ab} - 0.9382 \cdot z_{ab})^2 & \text{metric screws} \\ \frac{\pi}{4} \cdot (d_{ab} - 0.9743 \cdot z_{ab})^2 & \text{unified inch screws} \end{cases} = 352,503 \text{ [mm}^2\text{]}$$

$$A_{rab} = \frac{\pi}{4} \cdot (d_{ab} - 1.2990 \cdot z_{ab})^2 = 317,403 \text{ [mm}^2\text{]}$$

Component loads

Component specific temperature $T_C = +149 \text{ [}^\circ\text{C]}$ (equal to T_G if blank)

Reaction forces

R^{high}	x	y	z		R^{low}	x	y	z	
1	+52,533	+22,212	+627,58	[N]	1	-52,5326	-22,2117	+77,551	[N]
2	+52,533	+22,212	+627,58	[N]	2	-52,5326	-22,2117	+77,551	[N]

Material data

S_y = yield strength at temperature

S_u = ultimate tensile strength at temperature

S = maximum allowable stress value at temperature (used for code class 2 and 3)

S_m = design stress intensity value at temperature (used for code class 1)

PLATE BOLTS

Type of material

<input checked="" type="radio"/>	Ferritic steel
<input type="radio"/>	Austenitic steel

Material ISO Grade 8,8

$$\left. \begin{aligned} S_{ypb} &= 634 \\ S_{upb} &= 827 \end{aligned} \right\} [\text{N/mm}^2]$$

LEG PLATE

Material SA36

$$\left. \begin{aligned} S_{yl} &= 219 \\ S_{ul} &= 400 \\ S_l &= 115 \\ S_{ml} &= 133 \end{aligned} \right\} [\text{N/mm}^2]$$

FOOT PLATE

Material SA36

$$\left. \begin{aligned} S_{yf} &= 219 \\ S_{uf} &= 400 \\ S_f &= 115 \\ S_{mf} &= 133 \end{aligned} \right\} [\text{N/mm}^2]$$

ANCHOR BOLTS

Type of material

<input checked="" type="radio"/>	Ferritic steel
<input type="radio"/>	Austenitic steel

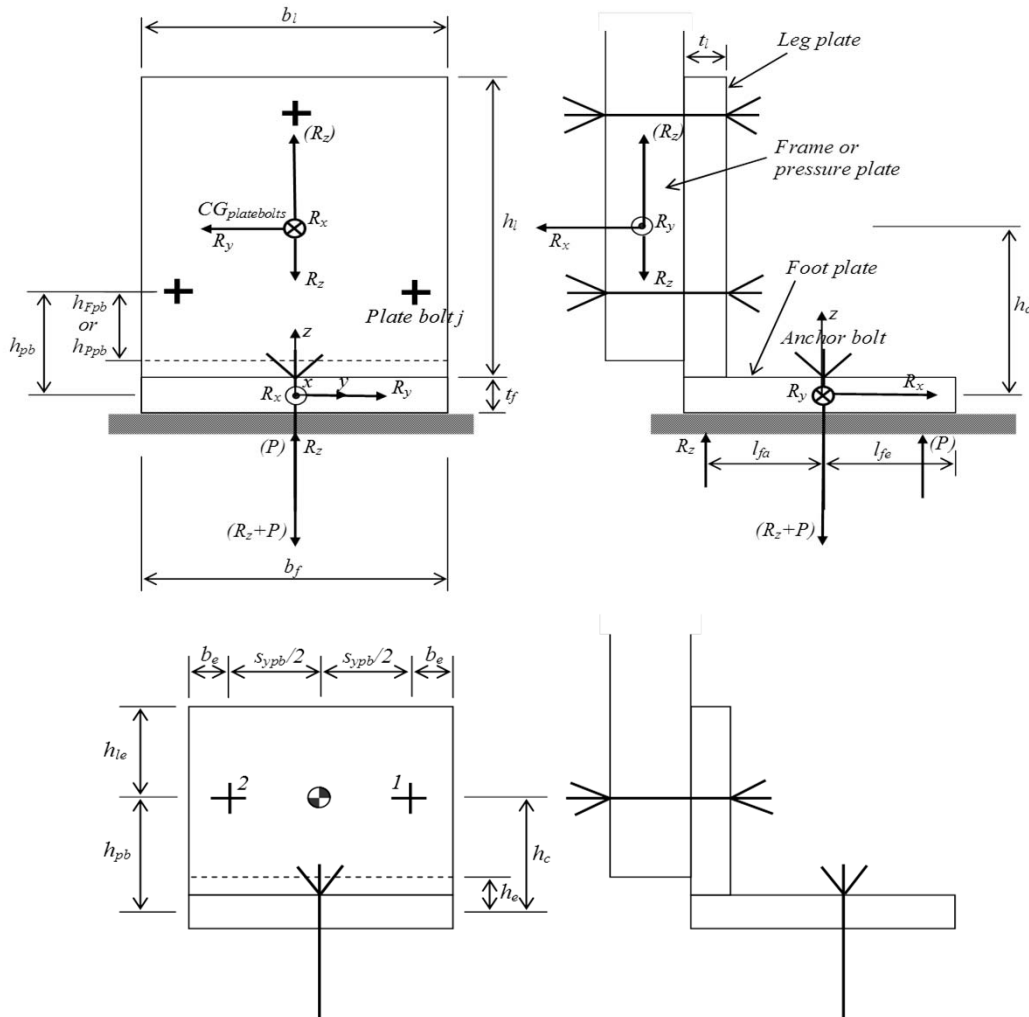
Material ISO Grade 8,8

$$\left. \begin{aligned} S_{yab} &= 634 \\ S_{uab} &= 827 \end{aligned} \right\} [\text{N/mm}^2]$$

SINGLE SUPPORT FOOT C ON THE FRAME PLATE

INPUT

Schematic pictures



Loads relating to negative R_z are in parenthesis. Thus R_z and (R_z) are not acting simultaneously.

$CG_{platebolts}$ =rotational center of gravity for the plate bolt group

The numbering of the plate bolts is related to a foot placed on the outside of the frame plate or on the inside of the pressure plate.

The foot is symmetrical about the xz -midplane.

All the bolts within the plate or anchor bolt group are identical.

Definitions

d_{bhl} = diameter of the plate bolt hole

N_{ab} = number of anchor bolts per foot

d_{bhf} = diameter of the anchor bolt hole

L_{bhf} = distance from the center of an anchor bolt hole to the nearest foot plate edge

L_{sbhf} = shortest distance between the centers of any two anchor bolt holes on a foot

SINGLE SUPPORT FOOT C ON THE FRAME PLATE

CRITERIA

S below is only applicable to Code Class 2 and 3 components. For analysis of Class 1 components, the value of S should be replaced by S_m in all equations below.

Plate bolts / Leg plate

STRESS CRITERION FOR THE PLATE BOLTS

Allowable tensile stress

$$F_{tpb} = \begin{cases} \frac{S_{upb}}{2} & \text{ferritic steel} \\ \frac{S_{upb}}{3.33} & \text{austenitic steel} \end{cases} = 413,5 \text{ [N/mm}^2\text{]}$$

Allowable shear stress

$$F_{vpb} = \begin{cases} \frac{0.62S_{upb}}{3} & \text{ferritic steel} \\ \frac{0.62S_{upb}}{5} & \text{austenitic steel} \end{cases} = 170,913 \text{ [N/mm}^2\text{]}$$

Combined tensile and shear stress

$\left(\frac{f_{tpbj}}{\kappa_{tpb} \cdot F_{tpb}} \right)^2 + \left(\frac{f_{vpbj}}{\kappa_{vpb} \cdot F_{vpb}} \right)^2 \leq 1$	Foot position		≤ 1	
	1	2		
	j=1	5,6E-05	5,6E-05	OK! OK!
	j=2	7,6E-05	7,6E-05	OK! OK!

Usage ratio

$u_{pb} = \max \left\{ \sqrt{\left(\frac{f_{tpbj}}{\kappa_{tpb} \cdot F_{tpb}} \right)^2 + \left(\frac{f_{vpbj}}{\kappa_{vpb} \cdot F_{vpb}} \right)^2} \right\} =$	Foot position	
	1	2
	0,0087	0,0087

STRESS CRITERIA FOR THE LEG PLATE

$\sigma_{eml} \leq \kappa_l \cdot S_l$	Foot position		\leq		
	1	2			
	0,37865	0,37865	152,95	[N/mm ²]	OK!
	0,37865	0,37865	152,95	[N/mm ²]	OK!

	Foot position					
$\sigma_{emb1} \leq 1.5\kappa_l \cdot S_l$	1		0,65562	\leq	229,425 [N/mm ²]	OK!
	2		0,65562	\leq	229,425 [N/mm ²]	OK!

BEARING STRESS CRITERION

$$f_{ppbj} \leq \min\left(\frac{L_{bhlj}}{2d_{pb}}, 1.5\right) \cdot S_{ul} \quad \text{(This criterion need not be evaluated for Service Level D, except that we need to ensure that } f_{ppbj} \leq 2.1S_{ul} \cdot \text{)}$$

Foot position

1	j=1	0,51749	\leq	250 [N/mm ²]	OK!
	j=2	0,6039	\leq	250 [N/mm ²]	OK!
2	j=1	0,51749	\leq	250 [N/mm ²]	OK!
	j=2	0,6039	\leq	250 [N/mm ²]	OK!

GEOMETRICAL LIMITATIONS

Minimum plate edge distance for bolts

$$\left\{ \begin{array}{l} \frac{L_{bhlj}}{d_{pb}} \geq \max\left\{\left(0.5 + 1.43 \frac{f_{ppbj}}{S_{ul}}\right), 1.2\right\} \quad \text{Service Levels: Design, A, B, C} \\ \frac{L_{bhlj}}{d_{pb}} \geq \left(0.5 + 1.2 \frac{f_{ppbj}}{S_{ul}}\right) \quad \text{Service Levels: D} \end{array} \right.$$

Foot position

1	j=1	1,25	\geq	max(0,50185 1,2)	OK!
	j=2	1,25	\geq	max(0,50216 1,2)	OK!
2	j=1	1,25	\geq	max(0,50185 1,2)	OK!
	j=2	1,25	\geq	max(0,50216 1,2)	OK!

Minimum bolt spacing

$$L_{sbhl} \geq 3d_{pb} \quad 50 \geq 48 \quad [\text{mm}] \quad \text{OK!}$$

Foot position

$$L_{sbhl} \geq \frac{2V_{pb}}{S_{ul} \cdot t_l} + \frac{d_{pb}}{2} \quad \begin{array}{l} 1 \quad 50 \geq 8,04831 \text{ [mm]} \quad \text{OK!} \\ 2 \quad 50 \geq 8,04831 \text{ [mm]} \quad \text{OK!} \end{array}$$

Geometrical restriction for column buckling of the leg plate

$$\frac{0.65\sqrt{12}h_{pb}}{t_l} = 3,50617 \leq 20 \quad \text{OK!}$$

Foot plate / Anchor bolts**STRESS CRITERIA FOR THE FOOT PLATE**

	Foot position				
$\sigma_{emf} \leq \kappa_f \cdot S_f$	1	0,03389	\leq	152,95 [N/mm ²]	OK!
	2	0,03389	\leq	152,95 [N/mm ²]	OK!

	Foot position				
$\sigma_{embf} \leq 1.5\kappa_f \cdot S_f$	1	0,03389	\leq	229,425 [N/mm ²]	OK!
	2	0,03389	\leq	229,425 [N/mm ²]	OK!

STRESS CRITERION FOR THE ANCHOR BOLTS

Allowable tensile stress

$$F_{tab} = \begin{cases} \frac{S_{uab}}{2} & \text{ferritic steel} \\ \frac{S_{uab}}{3.33} & \text{austenitic steel} \end{cases} = 413,5 \text{ [N/mm}^2\text{]}$$

Allowable shear stress

$$F_{vab} = \begin{cases} \frac{0.62S_{uab}}{3} & \text{ferritic steel} \\ \frac{0.62S_{uab}}{5} & \text{austenitic steel} \end{cases} = 170,913 \text{ [N/mm}^2\text{]}$$

Combined tensile and shear stress

	Foot position				
$\left(\frac{f_{tab}}{\kappa_{tab} \cdot F_{tab}}\right)^2 + \left(\frac{f_{vab}}{\kappa_{vab} \cdot F_{vab}}\right)^2 \leq 1$	1	8,4E-07	≤ 1		OK!
	2	8,4E-07	≤ 1		OK!

Usage ratio

	Foot position	
$u_{ab} = \sqrt{\left(\frac{f_{tab}}{\kappa_{tab} \cdot F_{tab}}\right)^2 + \left(\frac{f_{vab}}{\kappa_{vab} \cdot F_{vab}}\right)^2} =$	1	2
	0,00091	0,00091

BEARING STRESS CRITERION

$$f_{pab} \leq \min\left(\frac{L_{bhf}}{2d_{ab}}, 1.5\right) \cdot S_{uf}$$

(This criterion need not be evaluated for Service Level D, except that we need to ensure that $f_{pab} \leq 2 \cdot 1 S_{uf}$.)

Foot position

1	0,0679	≤	291,667 [N/mm ²]	OK!
2	0,0679	≤	291,667 [N/mm ²]	OK!

GEOMETRICAL LIMITATIONS

Minimum plate thickness

$$t_f \geq t_l \quad 35 \quad \geq \quad 35 \quad [\text{mm}] \quad \text{OK!}$$

Minimum plate edge distance for bolts

$$\left\{ \begin{array}{l} \frac{L_{bhf}}{d_{ab}} \geq \max\left\{ \left(0.5 + 1.43 \frac{f_{pab}}{S_{uf}} \right), 1.2 \right\} \\ \frac{L_{bhf}}{d_{ab}} \geq \left(0.5 + 1.2 \frac{f_{pab}}{S_{uf}} \right) \end{array} \right. \quad \begin{array}{l} \text{Service Levels: Design, A, B, C} \\ \text{Service Levels: D} \end{array}$$

Foot position

1	1,45833	≥	max(0,50024 1,2)	OK!
2	1,45833	≥	max(0,50024 1,2)	OK!

Minimum bolt spacing, applicable for the case of two or more anchor bolts only

$$L_{sbhf} \geq 3d_{ab} \quad \geq \quad [\text{mm}] \quad \text{Only one bolt!}$$

Foot position

$$L_{sbhf} \geq \frac{2V_{ab}}{S_{uf} \cdot t_f} + \frac{d_{ab}}{2} \quad \begin{array}{l} 1 \\ 2 \end{array} \quad \begin{array}{l} \geq \\ \geq \end{array} \quad \begin{array}{l} [\text{mm}] \\ [\text{mm}] \end{array} \quad \begin{array}{l} \text{Only one bolt!} \\ \text{Only one bolt!} \end{array}$$

SINGLE SUPPORT FOOT C ON THE PRESSURE PLATE

INPUT

Dimensions

PLATE BOLTS

Nominal diameter and thread pitch

$d_{pb} =$	16 [mm]	Type of bolt <input checked="" type="radio"/> Metric <input type="radio"/> Inch
$z_{pb} =$	2 [mm]	

Tensile stress, root, nominal and shear stress areas

$$A_{spb} = \begin{cases} \frac{\pi}{4} \cdot (d_{pb} - 0.9382 \cdot z_{pb})^2 & \text{metric screws} \\ \frac{\pi}{4} \cdot (d_{pb} - 0.9743 \cdot z_{pb})^2 & \text{unified inch screws} \end{cases} = 156,668 \text{ [mm}^2\text{]}$$

$$A_{rpb} = \frac{\pi}{4} \cdot (d_{pb} - 1.2990 \cdot z_{pb})^2 = 141,068 \text{ [mm}^2\text{]}$$

$$A_{npb} = \frac{\pi}{4} \cdot d_{pb}^2 = 201,062 \text{ [mm}^2\text{]}$$

Type of shear planes

<input type="radio"/> Threads not excluded from shear planes	$\Rightarrow A_{vpb} = A_{rpb}$	$\Rightarrow A_{vpb} = 201,062 \text{ [mm}^2\text{]}$
<input checked="" type="radio"/> Threads excluded from shear planes	$\Rightarrow A_{vpb} = A_{npb}$	

LEG PLATE

$$h_l = 57 \text{ [mm]} \quad d_{bhl} = 18 \text{ [mm]} \quad h_{pb} = 54,5 \text{ [mm]}$$

$$b_l = 90 \text{ [mm]} \quad s_{ypb} = 50 \text{ [mm]}$$

$$t_l = 35 \text{ [mm]} \quad h_{Fpb}, h_{Ppb} = 20 \text{ [mm]}$$

$$h_{ie} = h_l - h_{pb} + \frac{t_f}{2} = 20 \text{ [mm]}$$

$$h_e = h_{pb} - (h_{Fpb} \text{ or } h_{Ppb}) = 34,5 \text{ [mm]}$$

$$h_c = h_{pb} = 54,5 \text{ [mm]}$$

$$b_e = \frac{b_l - s_{ypb}}{2} = 20 \text{ [mm]}$$

Distance from the center of the plate bolt holes to the nearest leg plate edge

$$L_{bhl1} = L_{bhl2} = \min[b_e, h_{ie}, (h_{pb} - \frac{t_f}{2})] = 20 \text{ [mm]}$$

Shortest distance between the centers of the plate bolt holes

$$L_{sbhl} = s_{ypb} = 50 \text{ [mm]}$$

FOOT PLATE

$$\begin{array}{llll} l_{fa} = & 82,5 \text{ [mm]} & N_{ab} = & 1 \text{ [-]} & L_{bhf} = & 40 \text{ [mm]} \\ l_{fe} = & 40 \text{ [mm]} & d_{bhf} = & 32 \text{ [mm]} & L_{sbhf} = & \text{ [mm]} \\ b_f = & 100 \text{ [mm]} & & & & \\ t_f = & 35 \text{ [mm]} & & & & \end{array}$$

ANCHOR BOLTS

Nominal diameter and thread pitch

$$\begin{array}{ll} d_{ab} = & 24 \text{ [mm]} \\ z_{ab} = & 3 \text{ [mm]} \end{array}$$

Type of bolt
 Metric
 Inch

Tensile stress and root areas

$$A_{sab} = \begin{cases} \frac{\pi}{4} \cdot (d_{ab} - 0.9382 \cdot z_{ab})^2 & \text{metric screws} \\ \frac{\pi}{4} \cdot (d_{ab} - 0.9743 \cdot z_{ab})^2 & \text{unified inch screws} \end{cases} = 352,503 \text{ [mm}^2\text{]}$$

$$A_{rab} = \frac{\pi}{4} \cdot (d_{ab} - 1.2990 \cdot z_{ab})^2 = 317,403 \text{ [mm}^2\text{]}$$

Component loads

Component specific temperature $T_C = +149 \text{ [}^\circ\text{C]}$ (equal to T_G if blank)

Reaction forces

R^{high}	x	y	z		R^{low}	x	y	z	
4	0	+30,321	+763,41	[N]	4	0	-30,3208	+199,15	[N]
3	0	+30,321	+763,41	[N]	3	0	-30,3208	+199,15	[N]

Material data

S_y = yield strength at temperature

S_u = ultimate tensile strength at temperature

S = maximum allowable stress value at temperature (used for code class 2 and 3)

S_m = design stress intensity value at temperature (used for code class 1)

PLATE BOLTS

Type of material

<input checked="" type="radio"/>	Ferritic steel
<input type="radio"/>	Austenitic steel

Material ISO Grade 8,8

$$\left. \begin{aligned} S_{ypb} &= 634 \\ S_{upb} &= 827 \end{aligned} \right\} [\text{N/mm}^2]$$

LEG PLATE

Material SA36

$$\left. \begin{aligned} S_{yl} &= 219 \\ S_{ul} &= 400 \\ S_l &= 115 \\ S_{ml} &= 133 \end{aligned} \right\} [\text{N/mm}^2]$$

FOOT PLATE

Material SA36

$$\left. \begin{aligned} S_{yf} &= 219 \\ S_{uf} &= 400 \\ S_f &= 115 \\ S_{mf} &= 133 \end{aligned} \right\} [\text{N/mm}^2]$$

ANCHOR BOLTS

Type of material

<input checked="" type="radio"/>	Ferritic steel
<input type="radio"/>	Austenitic steel

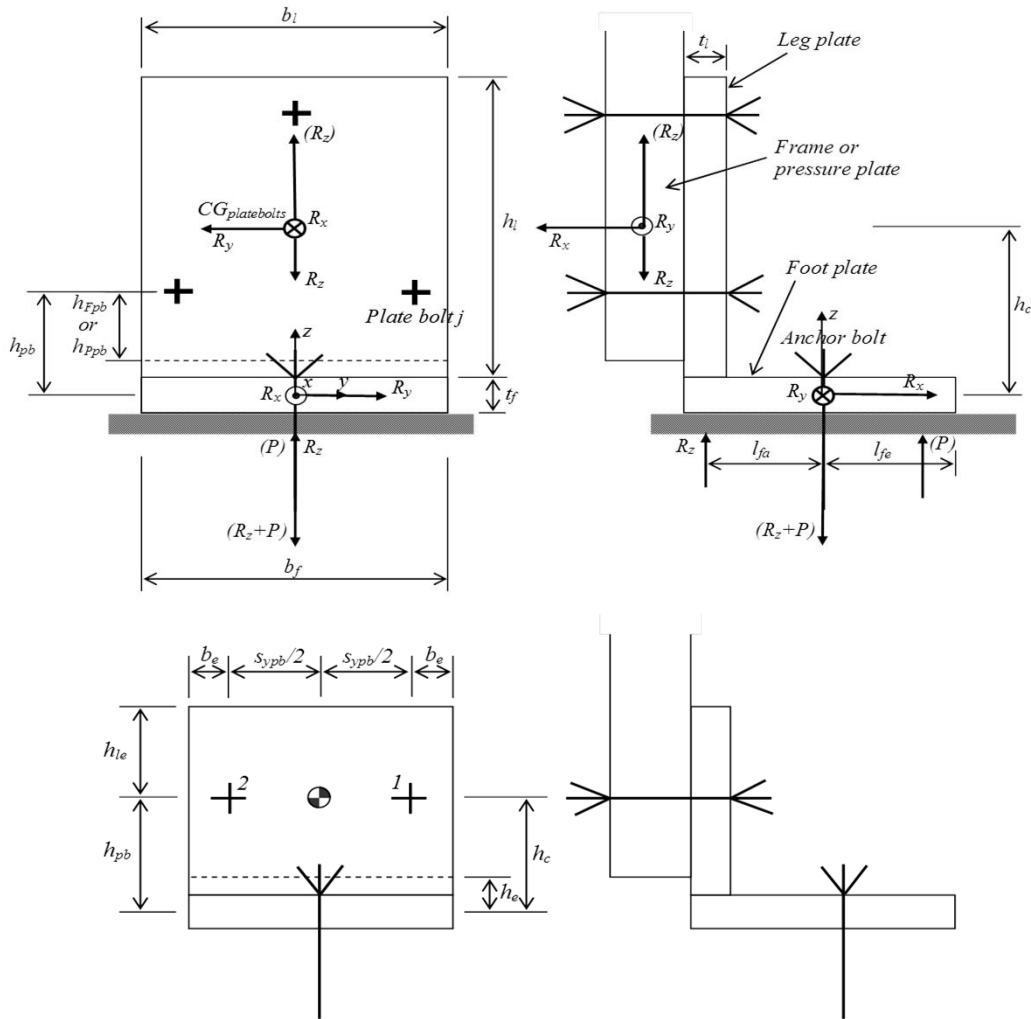
Material ISO Grade 8,8

$$\left. \begin{aligned} S_{yab} &= 634 \\ S_{uab} &= 827 \end{aligned} \right\} [\text{N/mm}^2]$$

SINGLE SUPPORT FOOT C ON THE PRESSURE PLATE

INPUT

Schematic pictures



Loads relating to negative R_z are in parenthesis. Thus R_z and (R_z) are not acting simultaneously.

$CG_{platebolts}$ =rotational center of gravity for the plate bolt group

The numbering of the plate bolts is related to a foot placed on the outside of the frame plate or on the inside of the pressure plate.

The foot is symmetrical about the xz-midplane.

All the bolts within the plate or anchor bolt group are identical.

Definitions

d_{bhl} = diameter of the plate bolt hole

N_{ab} = number of anchor bolts per foot

d_{bhf} = diameter of the anchor bolt hole

L_{bhf} = distance from the center of an anchor bolt hole to the nearest foot plate edge

L_{sbhf} = shortest distance between the centers of any two anchor bolt holes on a foot

SINGLE SUPPORT FOOT C ON THE PRESSURE PLATE

CRITERIA

S below is only applicable to Code Class 2 and 3 components. For analysis of Class 1 components, the value of S should be replaced by S_m in all equations below.

Plate bolts / Leg plate

STRESS CRITERION FOR THE PLATE BOLTS

Allowable tensile stress

$$F_{tpb} = \begin{cases} \frac{S_{upb}}{2} & \text{ferritic steel} \\ \frac{S_{upb}}{3.33} & \text{austenitic steel} \end{cases} = 413,5 \text{ [N/mm}^2\text{]}$$

Allowable shear stress

$$F_{vpb} = \begin{cases} \frac{0.62S_{upb}}{3} & \text{ferritic steel} \\ \frac{0.62S_{upb}}{5} & \text{austenitic steel} \end{cases} = 170,913 \text{ [N/mm}^2\text{]}$$

Combined tensile and shear stress

		Foot position		
		4	3	
$\left(\frac{f_{tpbj}}{\kappa_{tpb} \cdot F_{tpb}} \right)^2 + \left(\frac{f_{vpbj}}{\kappa_{vpb} \cdot F_{vpb}} \right)^2 \leq 1$	j=1	7,8E-05	7,8E-05	≤ 1 OK! OK! OK! OK!
	j=2	0,00011	0,00011	

Usage ratio

	Foot position	
	4	3
$u_{pb} = \max \left\{ \sqrt{\left(\frac{f_{tpbj}}{\kappa_{tpb} \cdot F_{tpb}} \right)^2 + \left(\frac{f_{vpbj}}{\kappa_{vpb} \cdot F_{vpb}} \right)^2} \right\} =$	0,01051	0,01051

STRESS CRITERIA FOR THE LEG PLATE

	Foot position				
	4	3			
$\sigma_{eml} \leq \kappa_l \cdot S_l$	4	0,46203	\leq	152,95 [N/mm ²]	OK!
	3	0,46203	\leq	152,95 [N/mm ²]	OK!

	Foot position				
$\sigma_{emb1} \leq 1.5\kappa_l \cdot S_l$	4		0,52869	\leq	229,425 [N/mm ²] OK!
	3		0,52869	\leq	229,425 [N/mm ²] OK!

BEARING STRESS CRITERION

$$f_{ppbj} \leq \min\left(\frac{L_{bhlj}}{2d_{pb}}, 1.5\right) \cdot S_{ul} \quad (\text{This criterion need not be evaluated for Service Level D, except that we need to ensure that } f_{ppbj} \leq 2.1S_{ul}.)$$

Foot position

4	j=1	0,62319	\leq	250 [N/mm ²]	OK!
	j=2	0,74113	\leq	250 [N/mm ²]	OK!
3	j=1	0,62319	\leq	250 [N/mm ²]	OK!
	j=2	0,74113	\leq	250 [N/mm ²]	OK!

GEOMETRICAL LIMITATIONS

Minimum plate edge distance for bolts

$$\left\{ \begin{array}{l} \frac{L_{bhlj}}{d_{pb}} \geq \max\left\{\left(0.5 + 1.43 \frac{f_{ppbj}}{S_{ul}}\right), 1.2\right\} \quad \text{Service Levels: Design, A, B, C} \\ \frac{L_{bhlj}}{d_{pb}} \geq \left(0.5 + 1.2 \frac{f_{ppbj}}{S_{ul}}\right) \quad \text{Service Levels: D} \end{array} \right.$$

Foot position

4	j=1	1,25	\geq	max(0,50223 1,2)	OK!
	j=2	1,25	\geq	max(0,50265 1,2)	OK!
3	j=1	1,25	\geq	max(0,50223 1,2)	OK!
	j=2	1,25	\geq	max(0,50265 1,2)	OK!

Minimum bolt spacing

$$L_{sbhl} \geq 3d_{pb} \quad 50 \geq 48 \quad [\text{mm}] \quad \text{OK!}$$

Foot position

$$L_{sbhl} \geq \frac{2V_{pb}}{S_{ul} \cdot t_l} + \frac{d_{pb}}{2} \quad \begin{array}{l} 4 \quad 50 \geq 8,05929 \text{ [mm]} \text{ OK!} \\ 3 \quad 50 \geq 8,05929 \text{ [mm]} \text{ OK!} \end{array}$$

Geometrical restriction for column buckling of the leg plate

$$\frac{0.65\sqrt{12}h_{pb}}{t_l} = 3,50617 \leq 20 \quad \text{OK!}$$

Foot plate / Anchor bolts**STRESS CRITERIA FOR THE FOOT PLATE**

	Foot position					
$\sigma_{emf} \leq \kappa_f \cdot S_f$	4	0,02548	\leq	152,95	[N/mm ²]	OK!
	3	0,02548	\leq	152,95	[N/mm ²]	OK!

	Foot position					
$\sigma_{embf} \leq 1.5\kappa_f \cdot S_f$	4	0,02548	\leq	229,425	[N/mm ²]	OK!
	3	0,02548	\leq	229,425	[N/mm ²]	OK!

STRESS CRITERION FOR THE ANCHOR BOLTS

Allowable tensile stress

$$F_{tab} = \begin{cases} \frac{S_{uab}}{2} & \text{ferritic steel} \\ \frac{S_{uab}}{3.33} & \text{austenitic steel} \end{cases} = 413,5 \text{ [N/mm}^2\text{]}$$

Allowable shear stress

$$F_{vab} = \begin{cases} \frac{0.62S_{uab}}{3} & \text{ferritic steel} \\ \frac{0.62S_{uab}}{5} & \text{austenitic steel} \end{cases} = 170,913 \text{ [N/mm}^2\text{]}$$

Combined tensile and shear stress

	Foot position				
$\left(\frac{f_{tab}}{\kappa_{tab} \cdot F_{tab}}\right)^2 + \left(\frac{f_{vab}}{\kappa_{vab} \cdot F_{vab}}\right)^2 \leq 1$	4	2,4E-07	\leq	1	OK!
	3	2,4E-07	\leq	1	OK!

Usage ratio

	Foot position	
	4	3
$u_{ab} = \sqrt{\left(\frac{f_{tab}}{\kappa_{tab} \cdot F_{tab}}\right)^2 + \left(\frac{f_{vab}}{\kappa_{vab} \cdot F_{vab}}\right)^2} =$	0,00049	0,00049

BEARING STRESS CRITERION

$$f_{pab} \leq \min\left(\frac{L_{bhf}}{2d_{ab}}, 1.5\right) \cdot S_{uf}$$

(This criterion need not be evaluated for Service Level D, except that we need to ensure that $f_{pab} \leq 2 \cdot 1 S_{uf}$.)

Foot position

4	0,0361	≤	333,333 [N/mm ²]	OK!
3	0,0361	≤	333,333 [N/mm ²]	OK!

GEOMETRICAL LIMITATIONS

Minimum plate thickness

$$t_f \geq t_l \quad 35 \quad \geq \quad 35 \quad [\text{mm}] \quad \text{OK!}$$

Minimum plate edge distance for bolts

$$\left\{ \begin{array}{l} \frac{L_{bhf}}{d_{ab}} \geq \max\left\{ \left(0.5 + 1.43 \frac{f_{pab}}{S_{uf}} \right), 1.2 \right\} \quad \text{Service Levels: Design, A, B, C} \\ \frac{L_{bhf}}{d_{ab}} \geq \left(0.5 + 1.2 \frac{f_{pab}}{S_{uf}} \right) \quad \text{Service Levels: D} \end{array} \right.$$

Foot position

4	1,66667	≥	max(0,50013 1,2)	OK!
3	1,66667	≥	max(0,50013 1,2)	OK!

Minimum bolt spacing, applicable for the case of two or more anchor bolts only

$$L_{sbhf} \geq 3d_{ab} \quad \geq \quad [\text{mm}] \quad \text{Only one bolt!}$$

Foot position

$$L_{sbhf} \geq \frac{2V_{ab}}{S_{uf} \cdot t_f} + \frac{d_{ab}}{2} \quad \begin{array}{l} 4 \\ 3 \end{array} \quad \begin{array}{l} \geq \\ \geq \end{array} \quad \begin{array}{l} [\text{mm}] \\ [\text{mm}] \end{array} \quad \begin{array}{l} \text{Only one bolt!} \\ \text{Only one bolt!} \end{array}$$

MIXTURE OF FILLET AND PARTIAL PENETRATION WELDS ON SINGLE SUPPORT FOOT ON THE FRAME PLATE

INPUT

Dimensions

WELD

$$a_{fw} = 5 \text{ [mm]}$$

$$a_{ppw} = 10 \text{ [mm]}$$

$$a_m = \min \{ a_{fw}, a_{ppw} \} = 5 \text{ [mm]}$$

LEG PLATE

$$b_l = 90 \text{ [mm]}$$

$$h_{pb} = 54,5 \text{ [mm]}$$

$$h_c = 54,5 \text{ [mm]}$$

$$t_l = 35 \text{ [mm]}$$

FOOT PLATE

$$l_{fa} = 67,5 \text{ [mm]}$$

$$t_f = 35 \text{ [mm]}$$

Component loads

Component specific temperature $T_C = +149 \text{ [}^\circ\text{C]}$ (equal to T_G if blank)

Reaction forces

R^{high}	x	y	z	R^{low}	x	y	z		
1	+52,533	+22,212	+627,58	[N]	1	-52,5326	-22,2117	+77,551	[N]
2	+52,533	+22,212	+627,58	[N]	2	-52,5326	-22,2117	+77,551	[N]

Material data

S_y = yield strength at temperature

S_u = ultimate tensile strength at temperature

S = maximum allowable stress value at temperature (used for code class 2 and 3)

S_m = design stress intensity value at temperature (used for code class 1)

LEG PLATE

$$\left. \begin{array}{l} S_{yl} = 219 \\ S_{ul} = 400 \\ S_l = 115 \\ S_{ml} = 133 \end{array} \right\} \text{ [N/mm}^2\text{]}$$

FOOT PLATE

$$\left. \begin{array}{l} S_{yf} = 219 \\ S_{uf} = 400 \\ S_f = 115 \\ S_{mf} = 133 \end{array} \right\} \text{ [N/mm}^2\text{]}$$

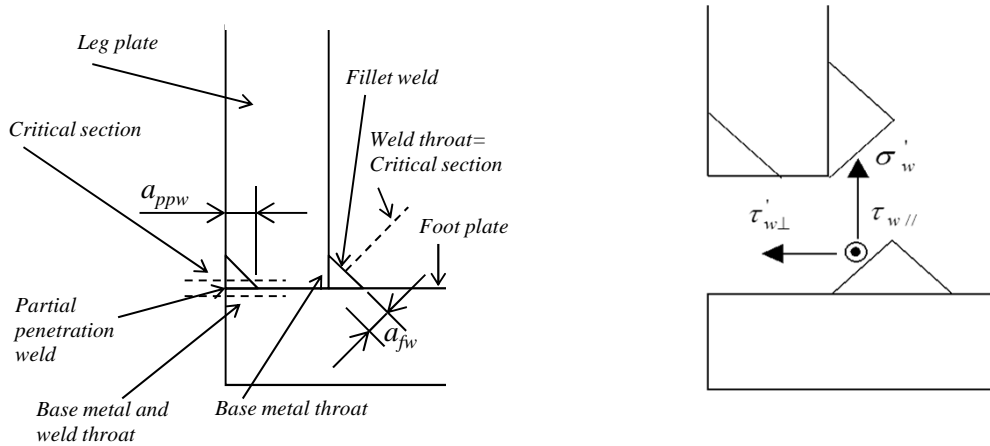
WELD

$$\left. \begin{aligned} S_{yw} &= \min(S_{yl}, S_{yf}) = & 219 \\ S_{uw} &= \min(S_{ul}, S_{uf}) = & 400 \\ S_w &= \min(S_l, S_f) = & 115 \\ S_{mw} &= \min(S_{ml}, S_{mf}) = & 133 \end{aligned} \right\} [\text{N/mm}^2]$$

MIXTURE OF FILLET AND PARTIAL PENETRATION WELDS ON SINGLE SUPPORT FOOT ON THE FRAME PLATE

INPUT

Schematic pictures



MIXTURE OF FILLET AND PARTIAL PENETRATION WELDS ON SINGLE SUPPORT FOOT ON THE FRAME PLATE

CRITERIA

Resultant stresses of the fillet weld part

	Foot position					
$\tau'_{ew} \leq 0.3\kappa_w \cdot S_{tw}$	1	0,48245	≤	159,6	[N/mm ²]	OK!
	2	0,48245	≤	159,6	[N/mm ²]	OK!

	Foot position					
$\tau'_{ewb} \leq 0.4\kappa_{wb} \cdot S_{yw}$	1	0,34114	≤	116,508	[N/mm ²]	OK!
	2	0,34114	≤	116,508	[N/mm ²]	OK!

Usage ratio of the partial penetration weld part

$$u_w = \sqrt{\left(\frac{\sigma_w}{\min\{0.3\kappa_{wt} \cdot S_{tw}, \kappa_{wc} \cdot (S_{mw} \text{ or } S_w)\}} \right)^2 + \left(\frac{\tau_w}{0.5\kappa_{ws} \cdot (S_{mw} \text{ or } S_w)} \right)^2} \leq 1$$

	Foot position					
	1	0,00329	≤	1		OK!
	2	0,00329	≤	1		OK!

FILLET WELDS ON SINGLE SUPPORT FOOT ON THE PRESSURE PLATE

INPUT

Dimensions

WELD

$$a = 5 \text{ [mm]}$$

LEG PLATE

$$b_l = 90 \text{ [mm]}$$

$$h_{pb} = 54,5 \text{ [mm]}$$

$$h_c = 54,5 \text{ [mm]}$$

$$t_l = 35 \text{ [mm]}$$

FOOT PLATE

$$l_{fa} = 82,5 \text{ [mm]}$$

$$t_f = 35 \text{ [mm]}$$

Component loads

Component specific temperature $T_C = +149 \text{ [}^\circ\text{C]}$ (equal to T_G if blank)

Reaction forces

R^{high}	x	y	z	R^{low}	x	y	z
4	0	+30,321	+763,41 [N]	4	0	-30,3208	+199,15 [N]
3	0	+30,321	+763,41 [N]	3	0	-30,3208	+199,15 [N]

Material data

S_y = yield strength at temperature

S_u = ultimate tensile strength at temperature

S = maximum allowable stress value at temperature (used for code class 2 and 3)

S_m = design stress intensity value at temperature (used for code class 1)

LEG PLATE

$$\left. \begin{array}{l} S_{yl} = 219 \\ S_{ul} = 400 \\ S_l = 115 \\ S_{ml} = 133 \end{array} \right\} \text{ [N/mm}^2\text{]}$$

FOOT PLATE

$$\left. \begin{array}{l} S_{yf} = 219 \\ S_{uf} = 400 \\ S_f = 115 \\ S_{mf} = 133 \end{array} \right\} \text{ [N/mm}^2\text{]}$$

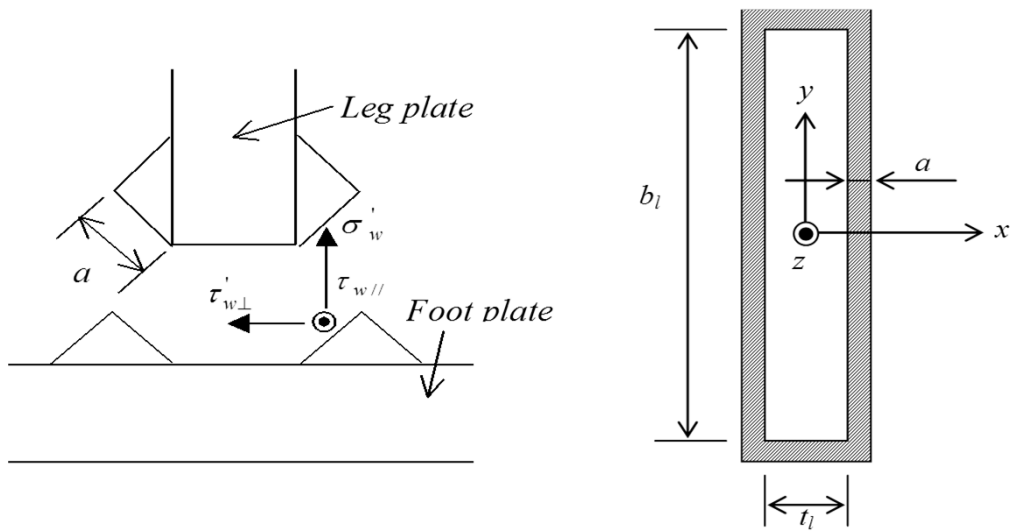
WELD

$$\left. \begin{aligned} S_{yw} &= \min(S_{yl}, S_{yf}) = & 219 \\ S_{uw} &= \min(S_{ul}, S_{uf}) = & 400 \\ S_w &= \min(S_l, S_f) = & 115 \\ S_{mw} &= \min(S_{ml}, S_{mf}) = & 133 \end{aligned} \right\} [\text{N/mm}^2]$$

FILLET WELDS ON SINGLE SUPPORT FOOT ON THE PRESSURE PLATE

INPUT

Schematic pictures



FILLET WELDS ON SINGLE SUPPORT FOOT ON THE PRESSURE PLATE

CRITERIA

	Foot position					
$\tau'_{ew} \leq 0.3\kappa_w \cdot S_{inv}$	4	0,63662	\leq	159,6	[N/mm ²]	OK!
	3	0,63662	\leq	159,6	[N/mm ²]	OK!
	Foot position					
$\tau'_{ewb} \leq 0.4\kappa_{wb} \cdot S_{ywb}$	4	0,45016	\leq	116,508	[N/mm ²]	OK!
	3	0,45016	\leq	116,508	[N/mm ²]	OK!

Customer:	Virdia B2X, LLC	SU Order No:	85480-40
Model :	TS6-MFG	Serial No:	30116-93128
Customer PO:	148	Item No:	HE-2853



Lifting Calculation for Plate Heat Exchanger

General

The following calculation is made to secure that the plate heat exchanger can be lifted with safety. The calculations are performed with respect to the following assumptions:

- The plate heat exchanger shall be lifted in vertical position. It means that the lift point and gravity center are included in a vertical line perpendicular to the ground.
- The plate heat exchanger is lifted from one of the corners of the frame plate and one of the corners in the pressure plate (two lifting points).
- The sling angle shall not be less than 60 degrees to the horizontal plane.
- The lifting load is increased by 2 times to account for lifting accelerations.

1 Input Data PHE

Estimated empty mass	$M = 165 \text{ kg}$
See principal figures 1 and 2 for definitions. The PHE is lifted in point A and D.	
Minimum vertical distance from upper edge of plates to lift point	$h = 300 \cdot \text{mm}$
Width between lifting lugs (y-direction)	$L_1 = 304 \cdot \text{mm}$
Width between lifting lugs (x-direction)	$L_2 = 75 \cdot \text{mm}$
Distance between frame plate and centre of gravity	$a = 43 \cdot \text{mm}$
Distance between pressure plate and centre of gravity	$b = 32 \cdot \text{mm}$
Distance between lifting point and centre of gravity (y-direction)	$c = 152 \cdot \text{mm}$
Distance between upper end of bolt hole and upper end of frame plate	$s_f = 8 \cdot \text{mm}$
Distance between upper end of bolt hole and upper end of pressure plate	$s_p = 8 \cdot \text{mm}$
Thickness of frame plate	$t_f = 30 \cdot \text{mm}$
Thickness of pressure plate	$t_p = 25 \cdot \text{mm}$

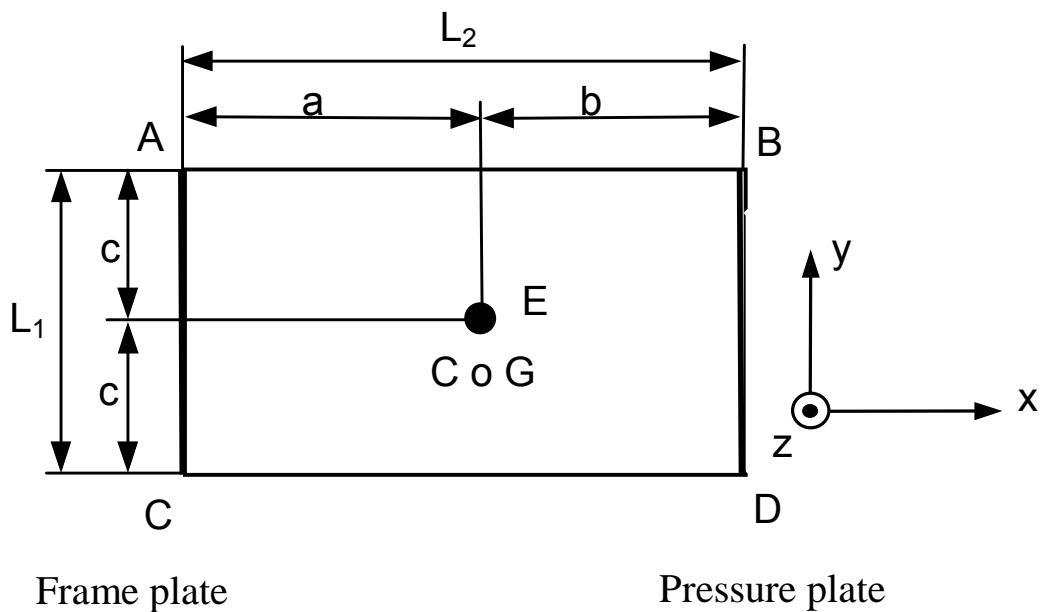


Figure 1: View from above

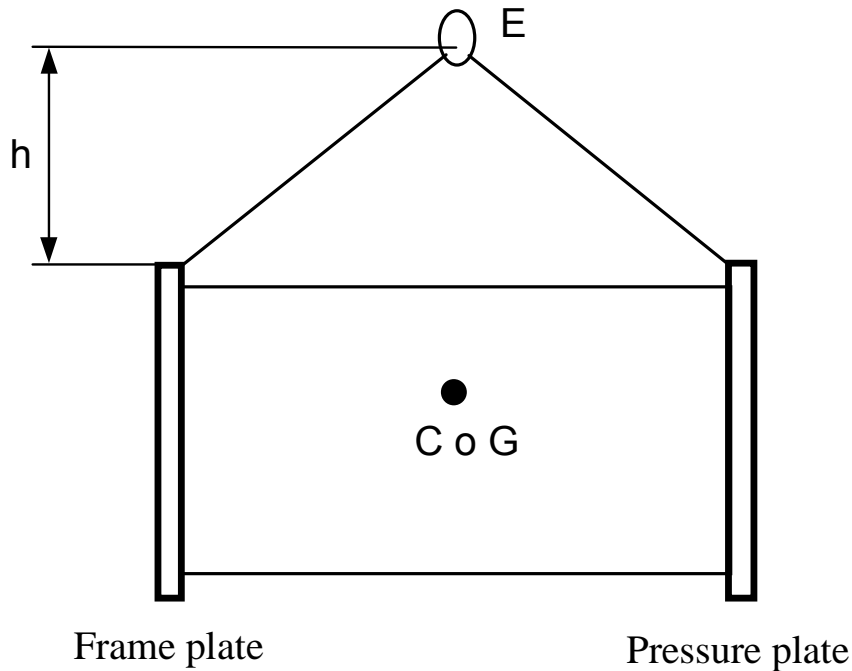


Figure 2: View from side

2 Lifting Calculation Constants

A dynamic factor is used to account for lifting accelerations.

Dynamic factor $F_d = 2.0$

3 Calculations

3.1 Design Load

The design load is calculated by multiplying the estimated weight by the dynamic factor.

Design load $W := F_d \cdot M$ $W = 330 \text{ kg}$

3.2 Distribution Reactions

Distribution of reaction loads are calculated by the area method.

Point A: $A := \frac{2 \cdot c \cdot b \cdot W}{L_1 \cdot L_2}$ $A = 141 \text{ kg}$

Point D: $D := \frac{2 \cdot c \cdot a \cdot W}{L_1 \cdot L_2}$ $D = 189 \text{ kg}$

3.3 Design Load on A and D

Reaction force at point A $R_{A1} := A$ $R_{A1} = 141 \text{ kg}$

Reaction force at point D $R_{D1} := D$ $R_{D1} = 189 \text{ kg}$

3.4 Angles

Sling AE to the horizontal plane $\Theta_{AE} := \text{atan}\left(\frac{h}{\sqrt{a^2 + c^2}}\right)$ $\Theta_{AE} = 62.2 \cdot \text{deg}$

Sling DE to the horizontal plane $\Theta_{DE} := \text{atan}\left(\frac{h}{\sqrt{b^2 + c^2}}\right)$ $\Theta_{DE} = 62.6 \cdot \text{deg}$

Sling angles should be not be less than 60 deg $\min(\Theta_{AE}, \Theta_{DE}) > 60 \text{deg} = 1$ *(1=true, 0=false)*

3.5 Minimum Sling Lengths

Sling AE $L_{AE} := \frac{h}{\sin(\Theta_{AE})}$ $L_{AE} = 339 \cdot \text{mm}$

Sling DE $L_{DE} := \frac{h}{\sin(\Theta_{DE})}$ $L_{DE} = 338 \cdot \text{mm}$

3.6 Maximum Sling Loads

Sling AE $AE := \frac{R_{A1}}{\sin(\Theta_{AE})}$ $AE = 159 \text{ kg}$

Sling DE $DE := \frac{R_{D1}}{\sin(\Theta_{DE})}$ $DE = 213 \text{ kg}$

Sling loads written as load vectors:

Load vector AE $v_{AE} := AE \cdot \begin{pmatrix} a \\ -c \\ h \end{pmatrix} \cdot \left[\begin{pmatrix} a \\ -c \\ h \end{pmatrix} \right]^{-1}$ $v_{AE} = \begin{pmatrix} 20 \\ -71 \\ 141 \end{pmatrix} \text{ kg}$

Load vector DE $v_{DE} := DE \cdot \begin{pmatrix} -b \\ c \\ h \end{pmatrix} \cdot \left[\begin{pmatrix} -b \\ c \\ h \end{pmatrix} \right]^{-1}$ $v_{DE} = \begin{pmatrix} -20 \\ 96 \\ 189 \end{pmatrix} \text{ kg}$

3.7 Choice of Components

Required Safe Working Load for slings

$$SWL_r := \max(AE, DE)$$

$$SWL_r = 213 \text{ kg}$$

Choice of Wire (1):

- Wire rope, galvanized, d = 10 mm

Allowable SWL

$$SWL_a = 2630 \text{ kg}$$

Actual SWL is greater than required:

$$SWL_a > SWL_r = 1 \quad (1=true, 0=false)$$

Note!

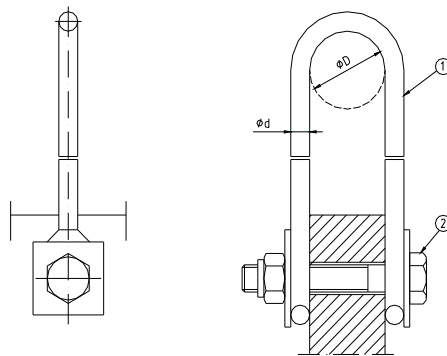
The bending diameter, D , of the wire rope shall not be less than $6d = 60 \text{ mm}$, otherwise the allowable SWL must be reduced.

Choice of Bolt (2):

- M20, Property Class 8.8

Root area of bolt

$$A_r = 220 \cdot \text{mm}^2$$



3.8 Stress Calculations

Allowable stresses are according to ASME SEC. III, Subsec. NF, Service Level A.

3.8.1 Stresses in Bolt

As the sling load are the same for all slings, the bolt stresses are calculated for sling AE.

The y- and z-component of the sling load expose the bolt for stress in shear.

$$\tau_b := \frac{\sqrt{(\nu AE_1)^2 + (\nu AE_2)^2} \cdot g}{2A_r} \quad \tau_b = 3.5 \cdot \text{MPa} \quad \tau_{b.all} = 165.0 \cdot \text{MPa}$$

Allowed tension stress is greater than actual: $\tau_{b.all} > \tau_b = 1$ (1=true, 0=false)

The x-component of the sling load expose the bolt for stress in tension.

$$\sigma_b := \frac{\nu AE_0 \cdot g}{A_r} \quad \sigma_b = 0.9 \cdot \text{MPa} \quad \sigma_{b.all} = 400.0 \cdot \text{MPa}$$

Allowed tension stress is greater than actual: $\sigma_{b.all} > \sigma_b = 1$ (1=true, 0=false)

3.8.2 Stresses in Plates

The z-component of the sling load in sling AE expose the frame plate for stress in shear in the weakest part of the plate.

$$\tau_{fp} := \frac{\nu AE_2 \cdot g}{s_f \cdot t_f} \quad \tau_{fp} = 5.8 \cdot \text{MPa} \quad \tau_{p.all} = 138.0 \cdot \text{MPa}$$

Allowed shear stress is greater than actual: $\tau_{p.all} > \tau_{fp} = 1$ (1=true, 0=false)

The z-component of the sling load in sling DE expose the pressure plate for stress in shear in the weakest part of the plate.

$$\tau_{pp} := \frac{\nu DE_2 \cdot g}{s_p \cdot t_p} \quad \tau_{pp} = 9.3 \cdot \text{MPa} \quad \tau_{p.all} = 138.0 \cdot \text{MPa}$$

Allowed shear stress is greater than actual: $\tau_{p.all} > \tau_{pp} = 1$ (1=true, 0=false)